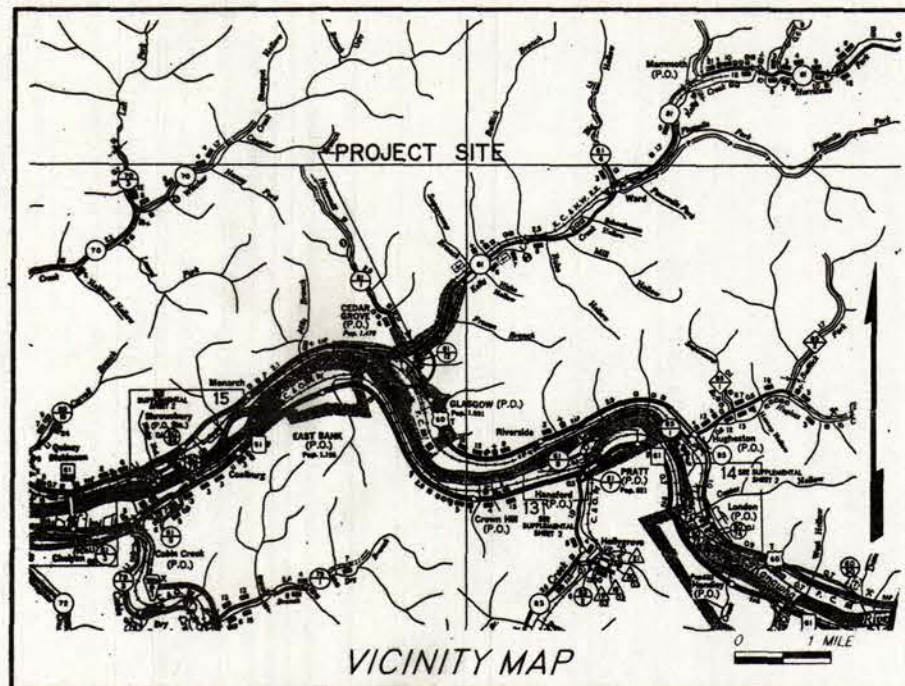


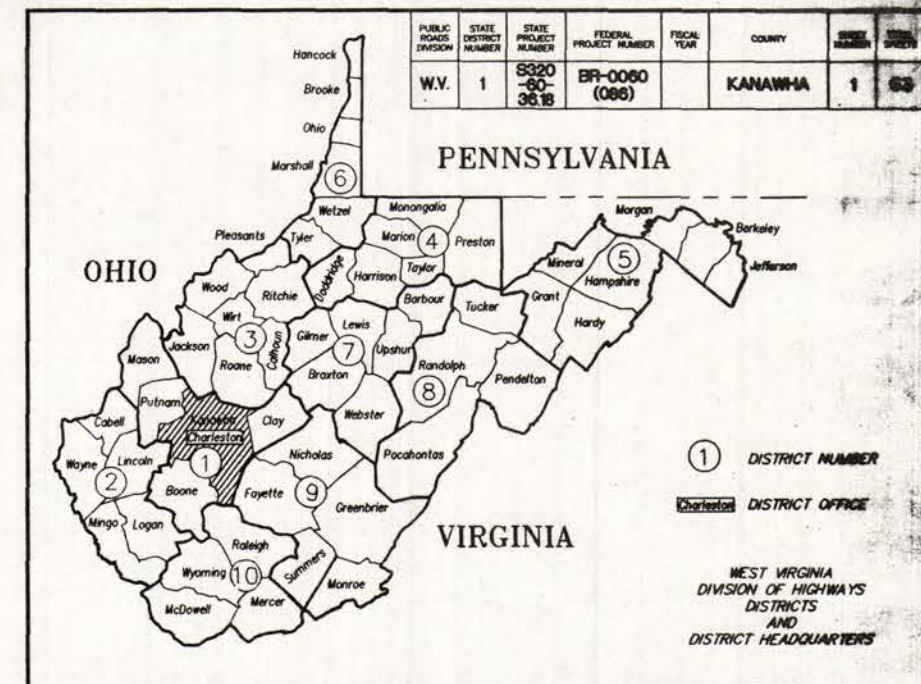


WEST VIRGINIA DEPARTMENT OF TRANSPORTATION DIVISION OF HIGHWAYS

PLANS FOR CONSTRUCTION OF STATE HIGHWAY

FEDERAL PROJECT NO. BR-0060(086)
STATE PROJECT NO. S320-60-36.18
ROUTE NO. U.S. ROUTE 60
CEDAR GROVE CORPORATION
KANAWHA COUNTY

KELLY CREEK BRIDGE



UTILITIES ENCOUNTERED:

APPALACHIAN POWER COMPANY
WV POWER GAS SERVICE
C&P TELEPHONE
TOWN OF CEDAR GROVE WATER & SEWER

	LENGTH	
STA. 64+00.00 TO STA. 68+00.00	400.00 LF	BRIDGE
STA. 68+00.00 TO STA. 70+07.70	242.30 LF	207.70
STA. 70+07.70 TO STA. 72+50.00	642.30 LF = 0.12 MI	207.70 LF = 0.04 MI
PROJECT TOTAL = 850 LF = 0.16 MI		

TYPE OF CONSTRUCTION:
GRADE, DRAIN, PAVEMENT,
BRIDGE # 3886, AND
SEWER RELOCATION

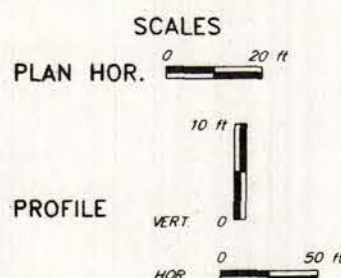
PLANS PREPARED BY:

Chapman Technical Group



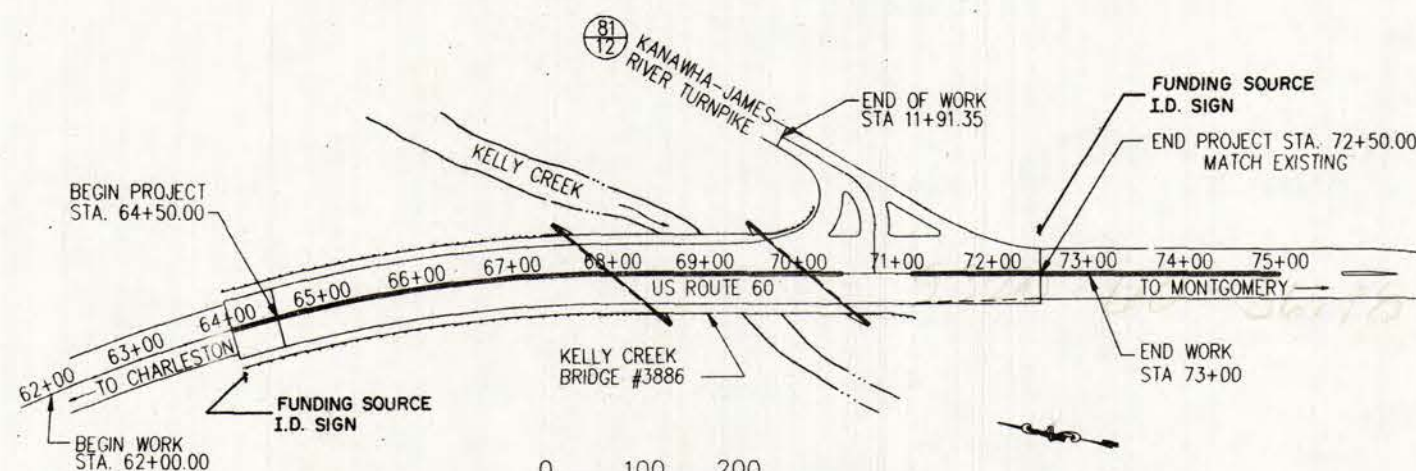
Harvey R. Chapman
REGISTERED PROFESSIONAL ENGINEER
3/10/93
DATE

NOTES:
STANDARD DETAIL BOOK VOL. I DATED
MARCH 1, 1993 & VOLUME II DATED
JAN. 1, 1994 SHALL APPLY TO THIS
PROJECT.



CONVENTIONAL SIGNS

---	STATE LINE
---	COUNTY LINE
---	CORPORATION LINE
---	PROPOSED R/W LINE
---	PROPERTY LINE
---	EXISTING FENCE
---	PROPOSED FENCE
---	EDGE OF STREAM
---	PROPOSED GUARD RAIL
---	EXISTING GUARD RAIL
---	RAILROAD
---	GAS LINE
---	WATER LINE
---	TELEPHONE LINE
---	ELECTRIC LINE
---	TELEPHONE POLE
---	POWER POLE
---	COMBINED POWER AND TELEPHONE POLE
---	TREE
---	SHRUB
---	RIGHT OF WAY MARKER



No.	DESCRIPTION
1	TITLE SHEET
2	TYPICAL SECTIONS
3	GENERAL NOTES AND TYPICAL SECTIONS
4	SUMMARY OF QUANTITIES
5	QUANTITY SUMMARY TABLES
6	SUPERELEVATION DATA
7	PAVEMENT DETAILS
8-18	STANDARD DETAILS (NOT USED)
19-21	SPECIAL DETAILS (NOT USED)
22-24	MAINTENANCE OF TRAFFIC
25-26	PLAN SHEETS
27	CENTERLINE PROFILE
28	PLAN-PROFILE LAYOUT OF INTERSECTION
29-53	BRIDGE NO. 3886
54	SANITARY SEWER RELOCATION
55-63	CROSS SECTIONS

REVISION NUMBER	SHEET NUMBER	REVISIONS	DATE	BY

I HERBY CERTIFY THAT THIS IS A CORRECT COPY OF THE
PLANS OF THE PROJECT BR-0060(086), S320-60-36.18

Harold L. Williams
BUSINESS MANAGER

DATE

PROJECT NO. BR - 0060(086), S320-60-36.18

RECOMMENDED: James E. Sothan
DIRECTOR OF STRUCTURES DIVISION

RECOMMENDED: Ralph J. Smith
DIRECTOR ROADWAY DESIGN DIVISION

RECOMMENDED: Norman A. White
CHIEF ENGINEER DEVELOPMENT

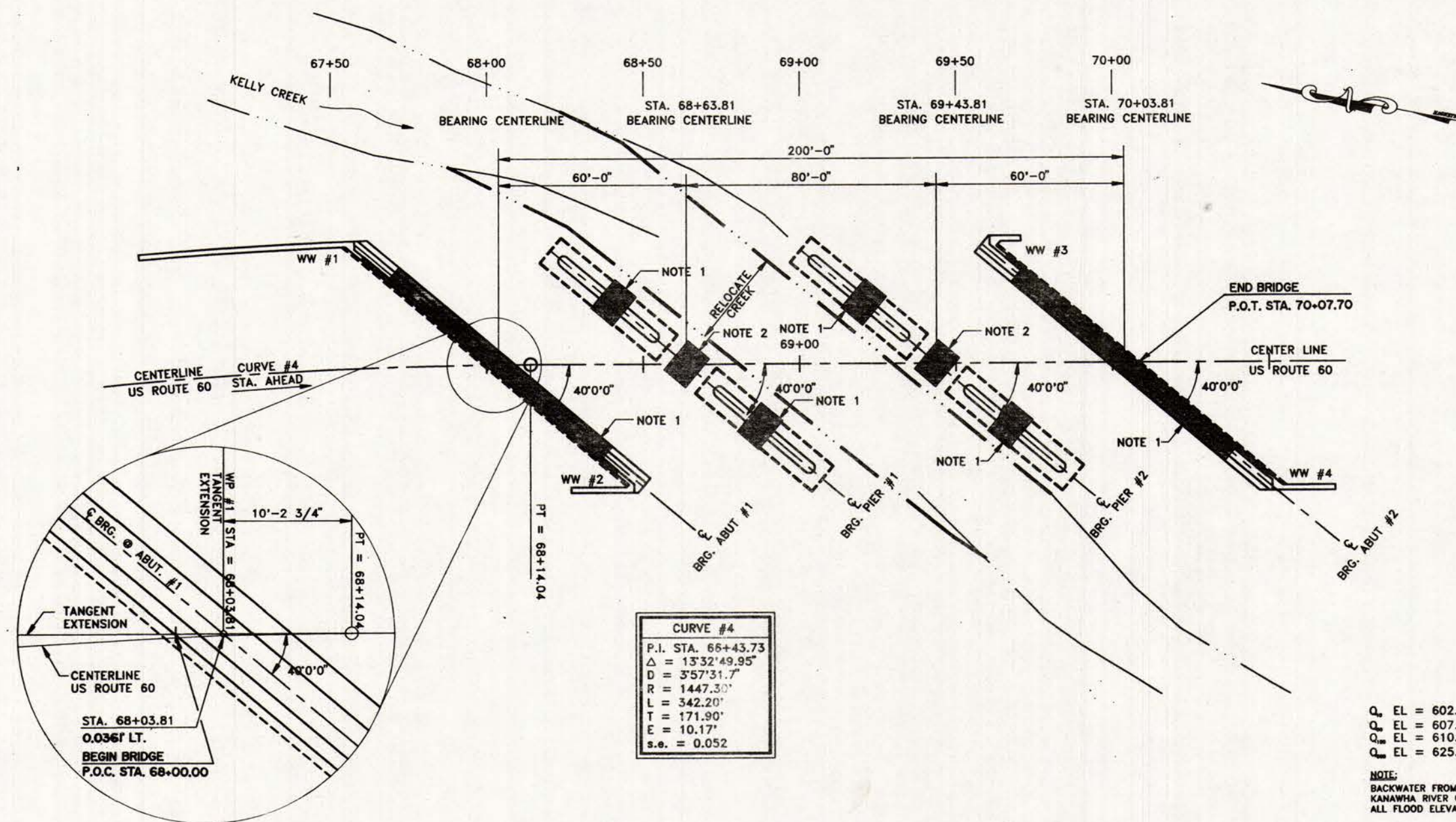
RECOMMENDED FOR APPROVAL: State Highway Engineer

APPROVED: Fred Van Hise
COMMISSIONER OF HIGHWAYS

APPROVED: _____
DIVISION ADMINISTRATOR

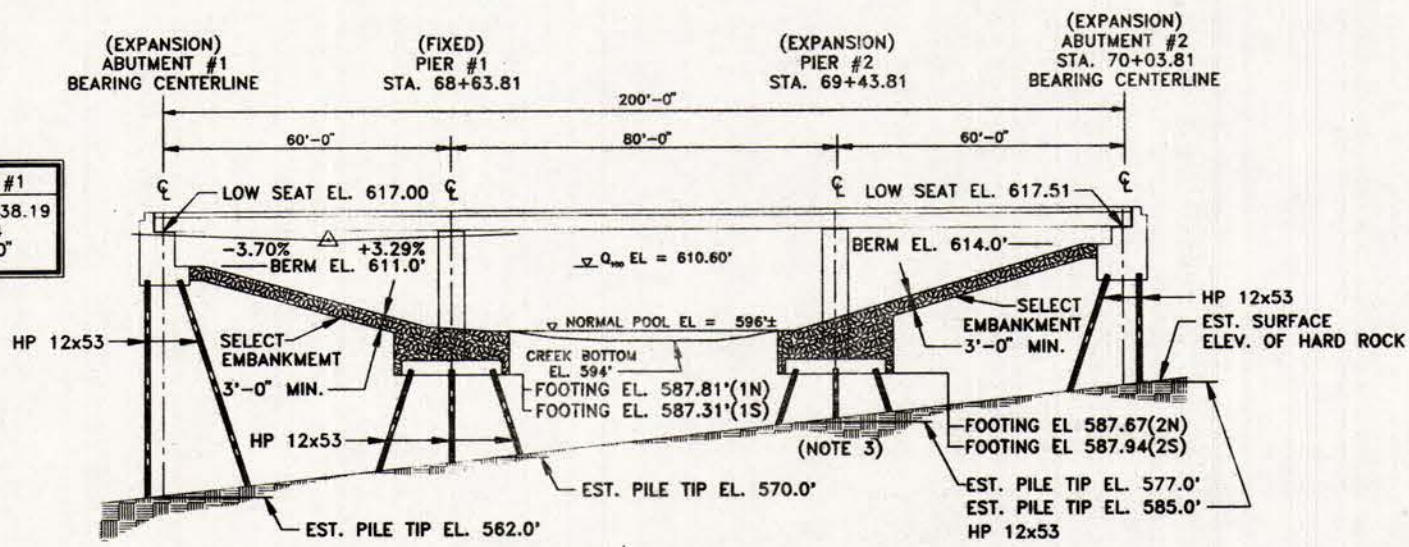
U.S. DEPARTMENT OF TRANSPORTATION
FEDERAL HIGHWAY ADMINISTRATION

LIST OF BRIDGE DRAWINGS	
SHEET NUMBER	DESCRIPTION
1	PLAN AND ELEVATION
2	STRUCTURE NOTES
3	ESTIMATED QUANTITIES
4	REMOVAL DETAILS & CONSTRUCTION STAGING
5	ABUTMENT 1
6	ABUTMENT 1 & WINGWALL #1
7	ABUTMENT 2
8	ABUTMENT 2 & WINGWALLS #2, 3 & 4
9	PIER 1N & 1S
10	PIER 2N & 2S
11	FRAMING PLAN
12	FRAMING DETAILS
13	BEARING DETAILS
14	DECK PLAN
15	DECK DETAILS
16	DECK DETAILS
17	DECK SLAB ELEVATIONS
18	EXPANSION JOINT
19	EXPANSION JOINT
20	APPROACH SLABS
21	REINFORCING BAR SCHEDULE
22	REINFORCING BAR SCHEDULE
23	SITUATION PLAN
24	BORE HOLE SECTIONS
25	BORE HOLE SECTIONS



CURVE #4
P.I. STA. 66+43.73
$\Delta = 13^\circ 32' 49.95"$
$D = 3^\circ 57' 31.7"$
$R = 1447.30'$
$L = 342.20'$
$T = 171.90'$
$E = 10.17'$
$s.e. = 0.052$

VERTICAL CURVE #1
P.V.I. STA. = 68+38.19
EL. = 616.34
L.V.C. = 630'-0"



Q₁ EL = 602.70'
 Q₂ EL = 607.30'
 Q₃ EL = 610.60'
 Q₄ EL = 625.90'

NOTE:
 BACKWATER FROM THE
 KANAWHA RIVER GOVERNS
 ALL FLOOD ELEVATIONS

- NOTES:
1. REMOVE EXISTING FOOTING IN ITS ENTIRETY, INCLUDING PILING. SEE "REMOVAL DETAILS AND CONSTRUCTION STAGING", SHEET 4, FOR SEQUENCE OF CONSTRUCTION.
 2. REMOVE EXISTING FOOTING TO 2'-0" BELOW FINAL GRADE AND LEAVE REMAINDER IN PLACE.
 3. BASED ON BOULDERS ENCOUNTERED DURING SUBSURFACE INVESTIGATION, PILE LOCATIONS AT PIER #2 SHALL BE PREDRILLED AND DRIVEN. MINIMUM PILE LENGTH IS 10 FEET.

- EXISTING FOOTINGS

REVISION NUMBER	SHEET NUMBER	REVISION	DATE	BY

SCALE: 1" = 20'

WEST VIRGINIA DEPARTMENT OF TRANSPORTATION
 DIVISION OF HIGHWAYS
 KELLY CREEK BRIDGE #3886 REPLACEMENT

PLAN AND ELEVATION

Chapman Technical Group

DATE: SEPTEMBER 1992

GOVERNING SPECIFICATIONS

THE WEST VIRGINIA DEPARTMENT OF HIGHWAYS STANDARD SPECIFICATIONS FOR ROADS AND BRIDGES AS CONTAINED IN THE BOOK ENTITLED "WEST VIRGINIA DEPARTMENT OF HIGHWAYS, STANDARD SPECIFICATIONS, ROADS AND BRIDGES", (ADOPTED 1993) AND AMENDED BY THE SUPPLEMENTAL SPECIFICATIONS (ADOPTED JANUARY 1, 1994), THE STANDARD SPECIFICATIONS FOR HIGHWAY BRIDGES, FIFTEENTH EDITION (1992), 1993 INTERIM, ADOPTED BY AASHTO ALONG WITH THE CONTRACT DOCUMENTS AND THE CONTRACT PLANS ARE THE GOVERNING SPECIFICATIONS APPLICABLE TO THIS PROJECT.

DESIGN

THE BRIDGE IS DESIGNED FOR HS25 LIVE LOADINGS AND PROVIDES FOR AN ADDITIONAL WEARING SURFACE OF 25 LBS. PER SQUARE FOOT OF ROADWAY. THE ADDITIONAL WEARING SURFACE IS NOT INCLUDED IN THIS CONTRACT. THE DESIGN IS IN ACCORDANCE WITH THE STANDARD SPECIFICATIONS FOR HIGHWAY BRIDGES, FIFTEENTH EDITION (1992), 1993 INTERIM, ADOPTED BY AASHTO. THE SEISMIC DESIGN IS IN ACCORDANCE WITH THE STANDARD SPECIFICATIONS FOR SEISMIC DESIGN OF HIGHWAY BRIDGES, 1983, 1991 INTERIM, ADOPTED BY AASHTO.

DESIGN UNIT STRESSES

CLASS "B" CONCRETE (PIERS AND ABUTMENTS) $f'_c = 3,000$ PSI
CLASS "K" CONCRETE (BRIDGE DECK AND PARAPETS) $f'_c = 4,500$ PSI
SUPERSTRUCTURE REINFORCING STEEL $f_y = 60,000$ PSI
SUBSTRUCTURE REINFORCING STEEL $f_y = 60,000$ PSI
STRUCTURAL STEEL, INCL. SPLICE PLATES, AASHTO M270 GR50 $f_y = 50,000$ PSI
PILING, MISC. FRAMING & DIAPHRAGMS AASHTO M270 GR36 $f_y = 36,000$ PSI

CONCRETE

WATER-REDUCING RETARDING ADMIXTURE IN ACCORDANCE WITH THE STANDARD SPECIFICATIONS WILL BE USED IN ALL CLASS "K" CONCRETE IN THE SUPERSTRUCTURE. RETARDER WILL NOT BE REQUIRED FOR TEMPERATURES BELOW 50 DEG. F. THE CONTRACTOR'S ATTENTION IS CALLED TO THE TEST REQUIREMENTS FOR RETARDER ADMIXTURE. PAYMENT SHALL BE INCLUDED IN ITEM 601-03.

CHAMFER ALL EXPOSED EDGES 3/4" BY 3/4" ON SUPERSTRUCTURE AND 1" BY 1" ON SUBSTRUCTURE UNLESS OTHERWISE NOTED.

CONCRETE IN ABUTMENT BACKWALLS ABOVE THE BRIDGE SEAT MAY BE PLACED WITHIN 2'-0" OF THE TOP OF THE BACKWALL PRIOR TO CONSTRUCTION OF THE SUPERSTRUCTURE DECK.

THE ABUTMENT, PARAPETS AND WINGWALLS SHALL BE CLASS 2, RUBBED FINISH, IN ACCORDANCE WITH SUBSECTION 601.11.2 OF THE STANDARD SPECIFICATIONS. BRIDGE DECK SHALL BE FINISHED IN ACCORDANCE WITH SUBSECTION 601.11.4 OF THE STANDARD SPECIFICATIONS.

WEEP DRAINS

WEEP DRAINS ARE TO BE OF A NON-RUSTING MATERIAL. THE COST OF THE WEEP DRAINS IS TO BE INCLUDED IN ITEM 601-02.

REINFORCING STEEL BARS

BARS SHALL CONFORM TO AASHTO M31 GRADE 60. ALL REINFORCING BARS IN DECK, PARAPETS AND THOSE DESIGNATED IN ABUTMENT BACKWALLS, BRIDGE SEATS, AND ABUTMENT PARAPETS SHALL BE EPOXY COATED.

THE CLEAR DISTANCE BETWEEN REINFORCING STEEL AND THE FACE OF CONCRETE SHALL BE AS FOLLOWS UNLESS OTHERWISE SHOWN ON THE DRAWINGS:

TOP OF SLABS	2-1/2 IN.
BOTTOM OF SLABS	1 IN.
FOOTINGS	3 IN.
ALL OTHER LOCATIONS	2 IN.

REINFORCEMENT UNDER BEARINGS SHALL BE PLACED TO AVOID INTERFERENCE WITH DRILLING OF ANCHOR BOLT HOLES.

PREFORMED JOINT FILLER

PREFORMED JOINT FILLER FOR VERTICAL JOINTS IN THE BARRIERS SHALL BE SPONGE RUBBER, TYPE 1, CONFORMING TO ARTICLE 708.1.1 OF THE STANDARD SPECIFICATIONS. THERE IS NO SEPARATE PAYMENT FOR PREFORMED JOINT FILLER AND ALL COSTS SHALL BE INCLUDED IN THE BID PRICE FOR ITEM 601-03.

ANCHOR BOLTS

ANCHOR BOLTS SHALL MEET THE REQUIREMENTS OF M183 AND SHALL HAVE HEX-NUT AND WASHER. WHEN ANCHOR BOLT GOES THROUGH SOLE PLATE, NUT IS TO BE 1/4" CLEAR. BURR THREADS AT FACE OF NUT.

ANCHOR BOLT HOLES SHALL BE DRILLED. THE ANCHOR BOLT HOLES SHALL BE FILLED WITH PORTLAND CEMENT NON-SHRINK GROUT IN ACCORDANCE WITH SECTION 715.5 OF THE STANDARD SPECIFICATIONS. THE GROUT SHALL BE PLACED IN STRICT ACCORDANCE WITH THE MANUFACTURER'S RECOMMENDATIONS. THERE IS NO SEPARATE PAYMENT FOR THE NON-SHRINK GROUT. ALL COSTS SHALL BE INCLUDED IN THE BID PRICE FOR ITEM 601-02.

BEARING AREAS

BRIDGE SEATS ON ABUTMENTS UNDER BEARING SHOES SHALL BE FINISHED TO TRUE PLANES AND ELEVATIONS.

EXCAVATION

EXCAVATION FOR ALL SUBSTRUCTURE FOUNDATIONS SHALL BE CLASSIFIED AS "STRUCTURE EXCAVATION" OR "WET EXCAVATION". THE LIMITS FOR EXCAVATION AT ABUTMENTS 1 & 2 AND PIERS 1 & 2 SHALL BE AS SHOWN ON THE PLANS.

THE ABUTMENT AND PIER CONCRETE SHALL BE POURED "IN THE DRY" USING METHODS SELECTED BY THE CONTRACTOR. NO DIRECT PAYMENT WILL BE MADE FOR COFFERDAMS IF THEY ARE REQUIRED. THE COST FOR "IN THE DRY" PLACEMENT SHALL BE INCLUDED IN THE BID PRICE FOR ITEM 212-02, WET EXCAVATION.

STRUCTURAL STEEL

THE LUMP SUM BID FOR ITEM 615-01 STEEL SUPERSTRUCTURE SHALL INCLUDE ALL STRUCTURAL STEEL COMPLETE IN PLACE INCLUDING ANCHOR BOLTS, BEARINGS, PREFORMED FABRIC PADS, DECK EXPANSION JOINTS, ALL PAINTING AND PREFORMED ELASTOMERIC STRIP SEALS. THE CONTRACTOR SHALL SUBMIT ALL FALSEWORK PLANS FOR REVIEW BY THE ENGINEER PRIOR TO BEGINNING OF WORK.

STRUCTURAL STEEL USED AS MAIN LOAD CARRYING MEMBERS SUBJECT TO TENSION OR STRESS REVERSAL SHALL MEET THE LONGITUDINAL CHARPY V-NOTCH TOUGHNESS TEST REQUIREMENTS. MAIN LOAD CARRYING MEMBERS FOR THIS STRUCTURE ARE LIMITED TO WIDE FLANGE BEAMS. M270 GRADE 50 STEEL, AT THE CONTRACTOR'S OPTION, MAY BE SUBSTITUTED FOR M270 GRADE 36 WITH THE DEPARTMENTS APPROVAL AND AT NO ADDITIONAL COST TO THE DEPARTMENT.

PILING

ALL HP 12x53 PILING SHALL HAVE A LOAD CAPACITY OF 70 TONS PER PILE. PILING SHALL BE DRIVEN TO REFUSAL WITH A POWER HAMMER DEVELOPING A MINIMUM CAPACITY OF 15,000 FOOT POUNDS PER BLOW. PRACTICAL REFUSAL IS DEFINED AS THE EQUIVALENT OF 20 BLOWS FOR ONE INCH OF PENETRATION. IF A LARGER HAMMER IS USED, THE NUMBER OF BLOWS IN THE LAST INCH OF PENETRATION MAY BE REDUCED IN DIRECT PROPORTION TO THE ENERGY RATING OF THE HAMMER BUT NO LESS THAN 12. PILE LOCATIONS AT PIERS #2N & 2S SHALL BE PRE-DRILLED AND DRIVEN TO A MINIMUM DEPTH OF 10' OR TO REFUSAL, WHICHEVER IS GREATER.

THE PREDRILLING HOLE SHALL BE 17 INCHES. THE VOID SHALL BE FILLED IN ACCORDANCE WITH SECTION 616.15 OF THE SPECIFICATIONS.

ALL PILING SHALL BE EQUIPPED WITH HARDENED PILE POINTS. PILE POINTS SHALL BE DFP HARD BITE H-776, MANUFACTURED BY DOUGHERTY FOUNDATION PRODUCTS, INC., HARD BITE HP-77600, MANUFACTURED BY ASSOCIATED PILE AND FITTING CORP., SP SERIES BY S.P.A. OR APPROVED EQUAL. THERE IS NO SEPARATE PAYMENT FOR PILE POINTS. ALL COSTS SHALL BE INCLUDED IN THE BID PRICE FOR ITEM 616-04.

BOLTS

DIAMETER OF HOLES SHALL BE 1/16-IN. LARGER THAN NOMINAL DIAMETER OF FASTENERS.

BEARING SHOES

SHOES SHALL BE SET SO THAT AT A TEMPERATURE OF 68 DEG. F. THEY ARE CENTERED UNDER FULL DEAD LOAD. SOLE PLATES SHALL BE BEVELED TO FIT BEAM GRADES WHEN BEAM GRADE EXCEEDS 1.0%.

PAINTING

SHOP AND FIELD PAINTING SHALL BE DONE IN ACCORDANCE WITH SECTION 615 OF THE STANDARD SPECIFICATIONS. THE COLOR FOR THE TOP COAT SHALL BE FROM FEDERAL STANDARD 595A.

FEDERAL STD. NO.

14223 (LT. GREEN)

ALL STEEL SURFACES, INCLUDING THOSE EMBEDDED IN CONCRETE SHALL RECEIVE A SHOP PRIMER. ALL AREAS WHICH WILL BE INACCESSIBLE AFTER THE CONCRETE DECK SLABS ARE PLACED SHALL BE PAINTED WITH THE SECOND AND THIRD COATS IN THE SHOP. DECK DRAINS SHALL NOT BE PAINTED BUT ARE TO BE GALVANIZED AS SHOWN IN THE DRAWINGS.

GALVANIZING

ANCHOR BOLTS SHALL BE GALVANIZED AS SHOWN ON THE DRAWINGS. GALVANIZING SHALL BE IN ACCORDANCE WITH ASTM A123 AND A153. COST IS TO BE INCLUDED IN THE PRICE PAID FOR STRUCTURAL STEEL. THE APPLICATION OF GALVANIZING SHALL BE 2 OUNCES PER SQUARE FOOT OF ACTUAL SURFACE.

BACKFILL

THE CONTRACTOR SHALL BACKFILL AROUND THE SUBSTRUCTURE AS SOON AS POSSIBLE AFTER REMOVAL OF FORMS AND FALSEWORK AND SHALL SLOPE SURFACES TO DRAIN.

HORIZONTAL CONTROL

ALL BEARINGS SHOWN ARE BASED ON AN ASSUMED NORTH. DIMENSIONS GIVEN ARE MEASURED HORIZONTALLY AND AT 68 DEG. F. UNLESS OTHERWISE NOTED.

ELEVATIONS

ELEVATIONS SHOWN REFER TO THE U.S. COAST AND GEODETIC SURVEYS BASED ON SEA LEVEL DATUM, 1929, 1957 ADJUSTMENT.

CONSTRUCTION JOINT KEYS

HORIZONTAL-KEYED CONSTRUCTION JOINTS MAY, AT THE CONTRACTOR'S OPTION, BE MADE A ROUGHENED CONSTRUCTION JOINT IN THE SUBSTRUCTURE.

REMOVAL OF EXISTING STRUCTURE

THE CONTRACTOR SHALL REMOVE AND DISPOSE OF THE EXISTING U.S. ROUTE 60 BRIDGE (BRIDGE #1575) IN ACCORDANCE WITH CONTRACT PLANS AND SECTION 203 OF THE SPECIFICATIONS AND ALL WORK SHALL BE PAID FOR UNDER ITEM 203-01, DISMANTLING STRUCTURES.

SELECT EMBANKMENT

THE SELECT EMBANKMENT SHALL BE SIZED FROM 12 TO 24 INCHES AND SHALL CONTAIN NO MORE THAN 15 PERCENT (BY VISUAL INSPECTIONS) OF OTHER SUITABLE UNCLASSIFIED MATERIAL.

ESTIMATED QUANTITIES

ITEM NO.	DESCRIPTION	UNIT	QUANTITY
203-01 (1)	DISMANTLING STRUCTURE	LS	1
212-01	STRUCTURE EXCAVATION	CY	1,338
212-02	WET EXCAVATION	CY	1,000
212-05	SELECT MATERIAL FOR BACKFILL	CY	-554
601-02	CLASS B CONCRETE	CY	1,846
601-03	CLASS K CONCRETE	CY	3,470
602-01	REINFORCING STEEL BARS	LB	101,291
602-02	EPOXY COATED REINFORCING STEEL BARS	LB	180,052
615-01	STEEL SUPERSTRUCTURE	LS	1
616-04 (HP 12X53)	STEEL BEARING PILES DRIVEN	LF	6,277
616-05 (HP 12X53)	STEEL BEARING PILES PREDRILLED AND DRIVEN	LF	538
639-01	CONSTRUCTION LAYOUT STAKES	LS	1

PUBLIC ROAD DIVISION	STATE DISTRICT NUMBER	STATE PROJECT NUMBER	FEDERAL PROJECT NUMBER	FISCAL YEAR	COUNTY	SHEET NUMBER	TOTAL SHEETS
W.V.	1	8320-60-36.18	BR-0080 (086)		KANAWHA	30	63

WEST VIRGINIA DEPARTMENT OF TRANSPORTATION
DIVISION OF HIGHWAYS
KELLY CREEK BRIDGE #3886 REPLACEMENT

STRUCTURE NOTES

Chapman Technical Group

SHEET NO.

2

REVISION NUMBER	30	CORRECT QUANTITIES	DATE	3-23-94	BY	JED
SCALE	NONE	DATE	SEPTEMBER 1992			

UNCOATED

EPOXY COATED

QUANTITY TABLE

PHASE III

3

PUBLIC ROADS DIVISION	STATE DISTRICT NUMBER	STATE PROJECT NUMBER	FEDERAL PROJECT NUMBER	FISCAL YEAR	COUNTY	DATE	BY
W.V.	1	8320-00-30.19	BR-0080 (086)		KANAWHA		

REMOVAL AND CONSTRUCTION SEQUENCE

PHASE I

THIS PHASE INVOLVES NON-BRIDGE WORK. SEE ROADWAY PLANS FOR DETAILS.

PHASE II

INSTALL TEMPORARY TRAFFIC BARRIER. (ANCHOR TO EXISTING DECK, REMOVE EXISTING MEDIAN ISLAND AS REQUIRED.)

ROUTE TRAFFIC TO NORTH HALF OF EXISTING SUPERSTRUCTURE TO LIMITS SHOWN. INSTALL TEMPORARY SHEETING, AS REQUIRED, ADJACENT TO THE PHASE II ROADWAY BEHIND EXISTING ABUTMENTS, REGRADE THE SOUTH HALF OF THE END SLOPES TO THE NEW CROSS SECTION AND REMOVE THE SOUTH HALF OF THE ABUTMENTS. REMOVE SOUTH HALF OF PIERS TO THE LIMITS SHOWN.

CONSTRUCT SOUTH HALF OF THE NEW BRIDGE SUBSTRUCTURE ELEMENTS TO THE LIMITS SHOWN, INCLUDING THE SOUTHWEST AND SOUTHEAST RETAINING WALLS. ERECT THE NEW STRUCTURAL STEEL GIRDERS AND DIAPHRAGMS AND CONSTRUCT THE NEW BRIDGE DECK AND BARRIER. INSTALL TEMPORARY TRAFFIC BARRIER ON THE NEWLY CONSTRUCTED BRIDGE DECK.

PHASE III

ROUTE TRAFFIC TO NEWLY CONSTRUCTED BRIDGE DECK. REMOVE TEMPORARY TRAFFIC BARRIER FROM EXISTING BRIDGE. REMOVE REMAINING EXISTING SUPERSTRUCTURE AND ABUTMENTS. REMOVE BALANCE OF EXISTING PIERS IN THEIR ENTIRETY EXCEPT AS NOTED. REGRADE NORTHERN HALF OF END SLOPES TO THE FINAL CROSS SECTION AND REMOVE REMAINING ABUTMENTS IN THEIR ENTIRETY.

COMPLETE CONSTRUCTION OF NEW ABUTMENTS AND PIERS, INCLUDING THE NORTHEAST AND NORTHWEST RETAINING WALLS. ERECT REMAINING NEW STRUCTURAL STEEL GIRDERS AND DIAPHRAGMS. CONSTRUCT NEW BRIDGE DECK AND BARRIER TO LIMITS SHOWN.

PHASE IV

INSTALL TEMPORARY TRAFFIC BARRIER ON THE NEWLY CONSTRUCTED NORTH HALF OF NEW BRIDGE. ROUTE ONE-WAY TRAFFIC TO EACH HALF OF BRIDGE USING THE OUTERMOST LANES.

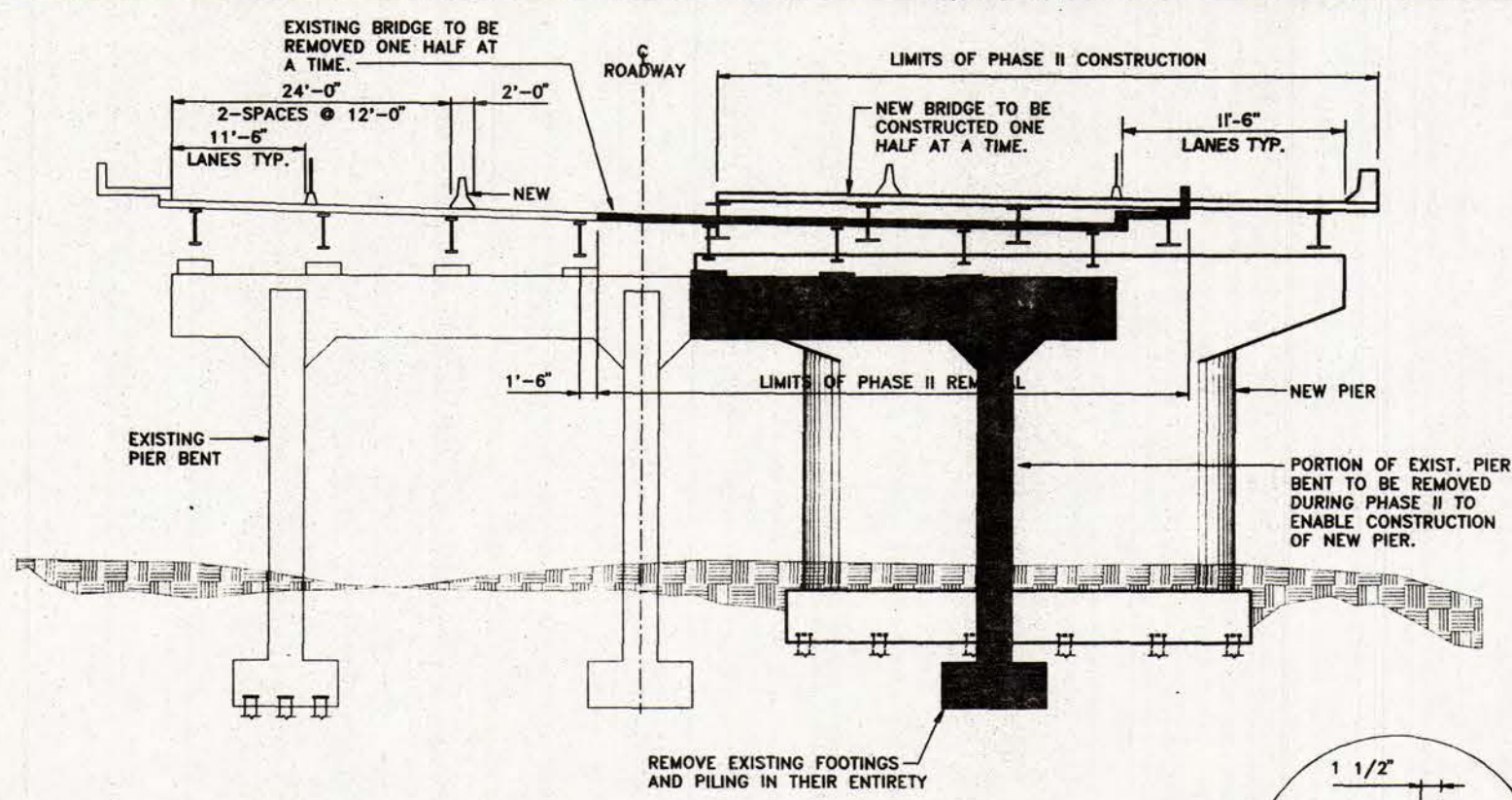
ERECT REMAINING NEW DIAPHRAGMS. CONSTRUCT REMAINING NEW BRIDGE AND MEDIAN. REMOVE TEMPORARY TRAFFIC BARRIERS ON BOTH HALVES OF BRIDGE. REPAIR DECK AS REQUIRED. RESUME NORMAL TRAFFIC. PHASE IV DIAPHRAGMS MAY BE SET INTO PLACE DURING PHASE III CONSTRUCTION AS LONG AS THEIR END CONNECTIONS ARE FREE TO ROTATE DURING PLACEMENT OF CONCRETE. FINAL FITUP OF THESE DIAPHRAGMS SHALL OCCUR DURING PHASE IV CONSTRUCTION.

NOTES:

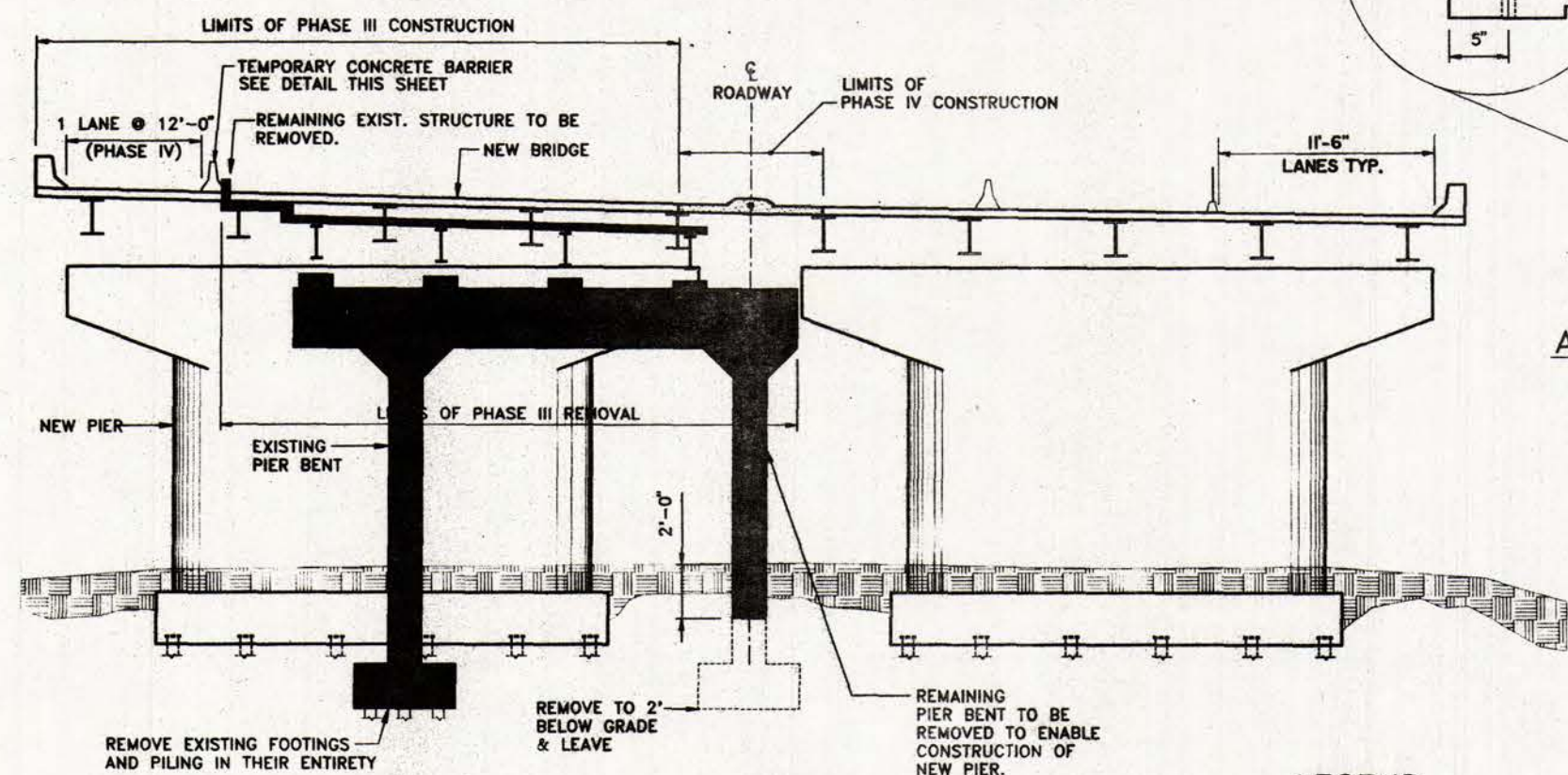
1. THERE IS NO SEPARATE PAYMENT FOR TEMPORARY SHEETING REQUIRED BY THE CONSTRUCTION AND ALL COSTS SHALL BE INCLUDED IN THE BID PRICE FOR ITEM 601-02.
2. ALL ANCHORS SHALL BE 1" DIAMETER, HIGH STRENGTH, THRU BOLTS OR APPROVED RESIN ANCHORS SPACED AT 24" CENTERS. WHEN RESIN ANCHORS ARE USED, THEY MUST BE EMBEDDED A MINIMUM OF 6" INTO FIRM CONCRETE. THE ANCHORS SHALL BE PLACED ON THE ROADWAY SIDE OF THE BARRIER.
3. THE BRIDGE DECK SURFACE AREA ON WHICH THE PRECAST CONCRETE BARRIER SEGMENTS WILL REST SHALL BE CLEARED OF ALL LOOSE SAND, GRAVEL, DIRT, AND DEBRIS.

ANY IRREGULARITIES IN THE BRIDGE DECK AREA, UNLESS JUDGED BY THE ENGINEER TO BE INCONSEQUENTIAL, SHALL BE LEVELED WITH GROUT AND/OR ASPHALT.

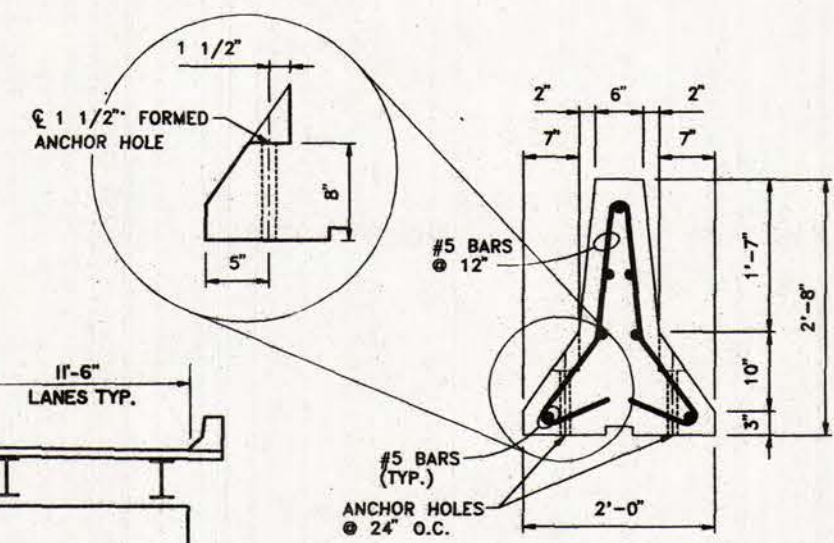
ASPHALT ROLL ROOFING SHALL BE PLACED ON THOSE BRIDGE DECK AREAS, AS JUDGED BY THE ENGINEER, TO HAVE A SURFACE ROUGHNESS WHICH WOULD INHIBIT FRICTION CONTACT BETWEEN BARRIER SEGMENTS AND DECK. THE COST OF THIS ITEM SHALL BE INCLUDED IN ITEM 636-17 "TEMPORARY BARRIER".



PIERS 1 AND 2 FOR PHASE II CONSTRUCTION



PIERS 1 AND 2 FOR PHASE III CONSTRUCTION



ANCHORED TEMPORARY CONCRETE BARRIER

NO SCALE - FOR ADDITIONAL DETAIL, SEE TEMPORARY BARRIER-STANDARD DETAIL SHEET.

LEGEND

■ DENOTES STRUCTURE TO BE REMOVED

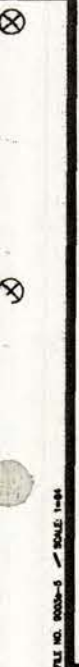
REVISION NUMBER	SHEET NUMBER	REVISIONS	DATE	BY
SCALE: NONE	DATE: SEPTEMBER 1992			

WEST VIRGINIA DEPARTMENT OF TRANSPORTATION
DIVISION OF HIGHWAYS
KELLY CREEK BRIDGE #3006 REPLACEMENT

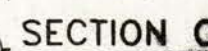
REMOVAL DETAILS AND CONSTRUCTION STAGING

Chapman Technical Group

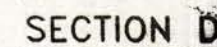
PROFILE		BT	DT	AN	BT	DATE
SAFETY BOOK	SURVEYED			SURVEYED		
NO	PICTURED			PICTURED		
	CHECKED			INDEXES CHECKED		
	IS A. NOTED					
	STRUCT. NOTATIONS CHD.			STRUCT. NOTATIONS CHD.		



SECTION A



SECTION B



ABUTMENT NO. 1

Chapman Technical Group

15

60'-0"

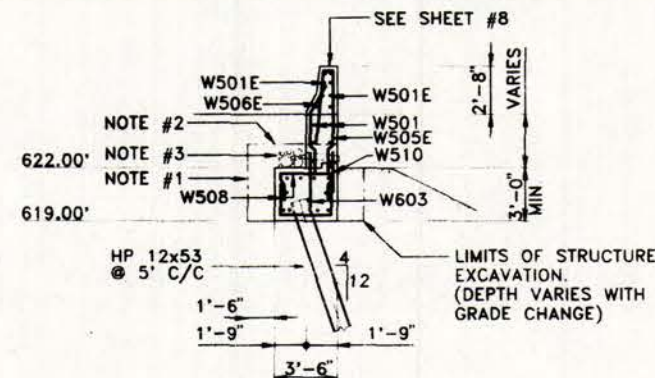
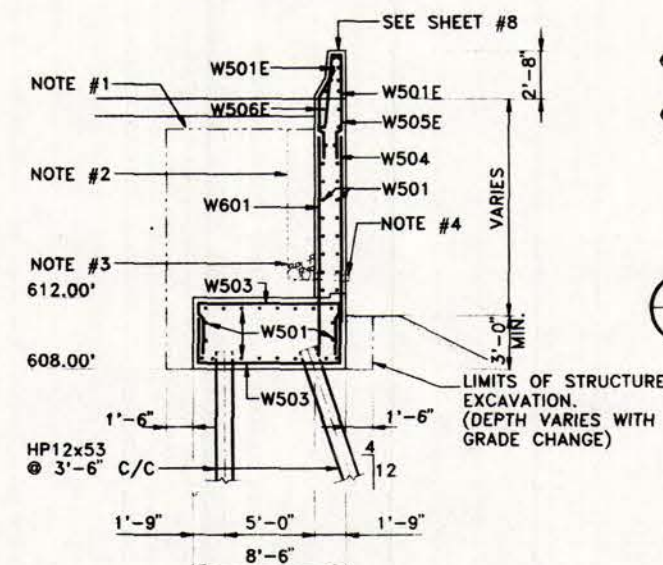


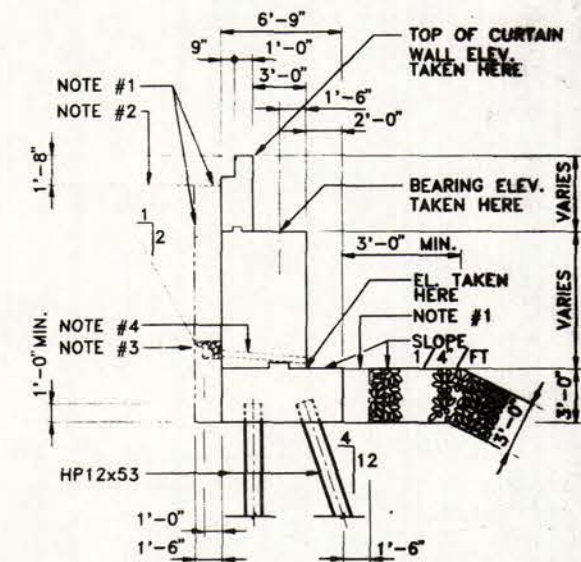
Diagram illustrating the cross-section of a bridge structure, showing various components and dimensions:

- Structural Components:** W501E, W506E, W602, W505E, W501, W509, W507, W501, HP12x53 @ 3'-6" C/C.
- Dimensions:**
 - Vertical dimensions: 616.00', 613.00', 1'-6", 1'-9", 4'-0", 7'-6".
 - Horizontal dimensions: 1'-6", 1'-9", 4'-0", 1'-9".
 - Other dimensions: 2'-8", 3'-0" MIN., 1'-6".
- Notes:**
 - NOTE #1
 - NOTE #2
 - NOTE #3
 - NOTE #4
- Labels:** SEE SHEET #8, LIMITS OF STRUCTURE EXCAVATION. (DEPTH VARIES WITH GRADE CHANGE).

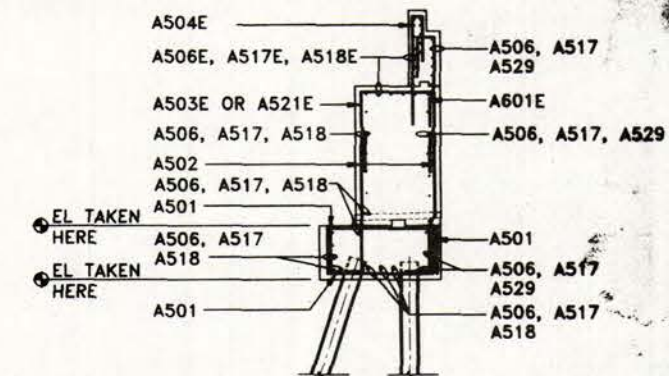
SCALE: $3/16" = 1'-0"$



SCALE: $3/16" = 1'-0"$



SCALE: $3/16" = 1'-0"$



6 SCALE: 3/16" = 1'-0"

- ▼ DENOTES PILES BATTERED @4:12

WING WALL #1

16

JEL
BY _____

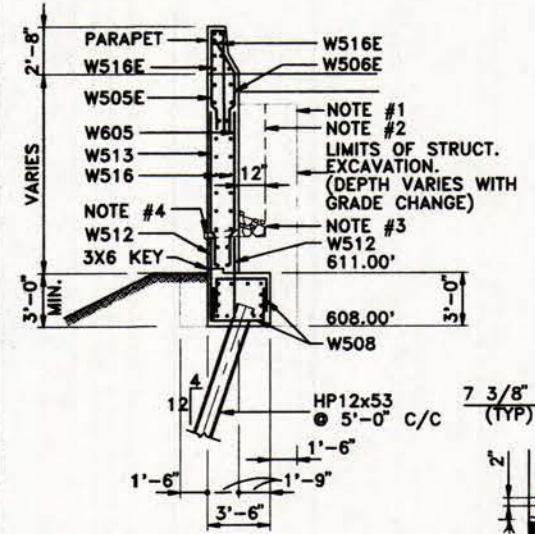
REVISION NUMBER SHEET NUMBER NEWSHOPS DATE BY

SCALE: AS SHOWN DATE: SEPTEMBER 1967

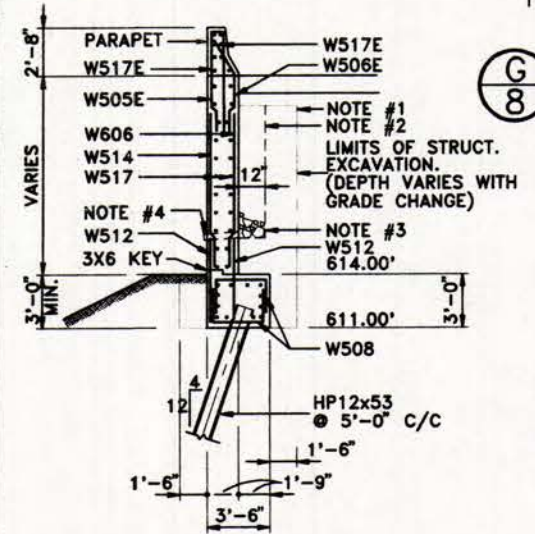
SCALE: AS SHOWN

DATE: SEPTEMBER 1961

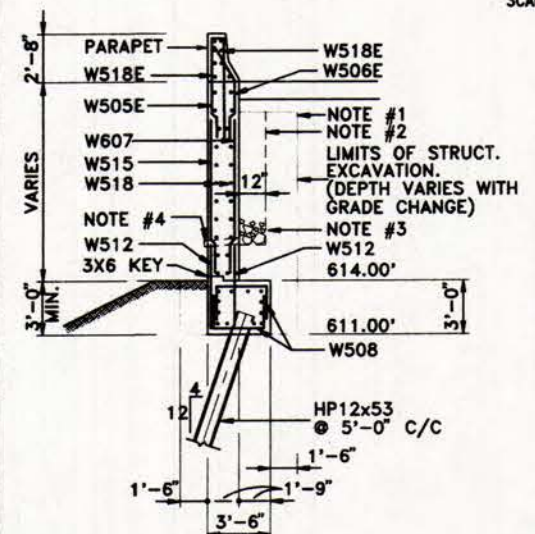
PUBLIC ROAD DISTRICT	STATE PROJECT NUMBER	FEDERAL PROJECT NUMBER	FISCAL YEAR	COUNTY	SHEET NUMBER	TOTAL SHEETS
W.V. 1	8320-60-36.18	EP-0080 (086)		KANAWHA	36	63



SECTION A
SCALE: 3/16" = 1'-0"

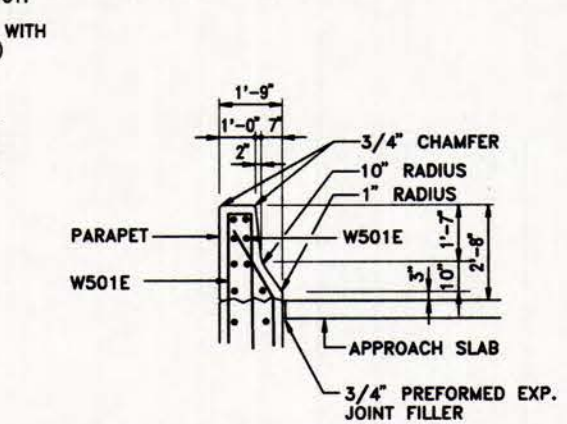


SECTION B
SCALE: 3/16" = 1'-0"

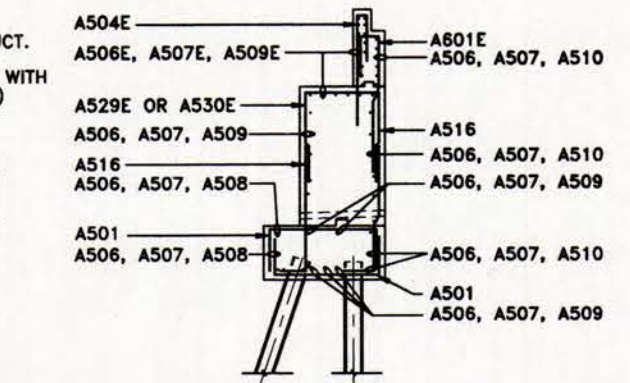


SECTION C
SCALE: 3/16" = 1'-0"

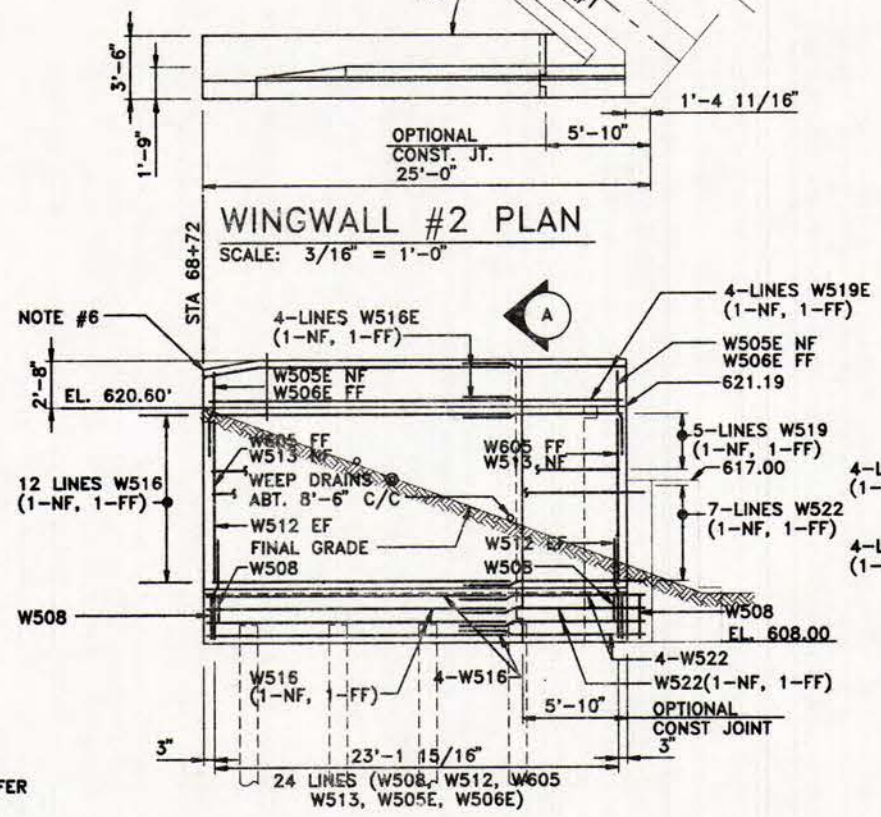
8 MEDIAN AT TOP OF CURTAIN WALL
SCALE: 1" = 1'-0"



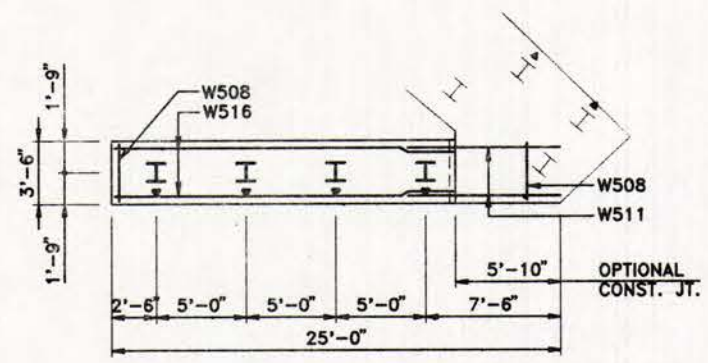
TYPICAL PARAPET SECTION
SCALE: 3/8" = 1'-0"



8 SECTION THRU ABUTMENT #2
SCALE: 3/16" = 1'-0"



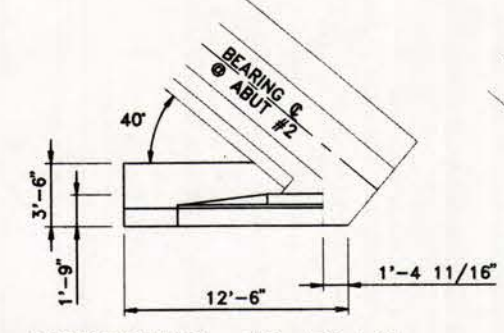
WINGWALL #2 ELEVATION
SCALE: 3/16" = 1'-0" (PHASE II CONSTRUCTION)



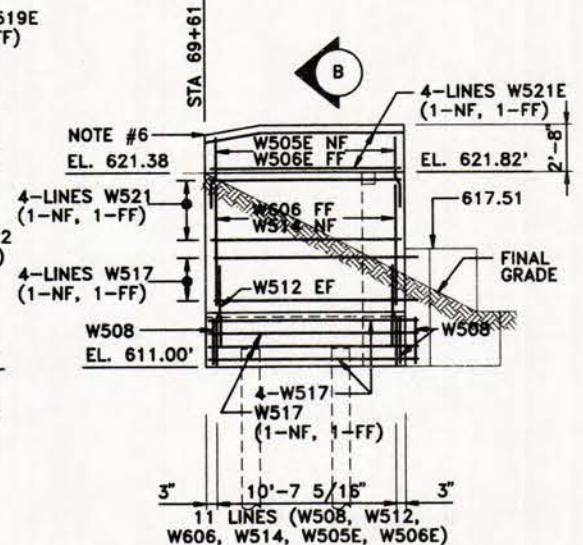
WINGWALL #2 FOOTING PLAN
SCALE: 3/16" = 1'-0"

- LEGEND**
- NF = NEAR FACE
 - FF = FAR FACE
 - EF = EACH FACE
- NOTE:**
- LIMITS OF STRUCTURE EXCAVATION
 - LIMITS OF SELECT EMBANKMENT MATERIAL FOR BACKFILLING
 - STACK STONES AROUND WEEP HOLE TO PREVENT LOSS OF BACKFILL (COST INCLUDED IN ITEM 212-50)
 - 4" WEEP DRAIN, SLOPE 1/4" PER FOOT
 - ALL PILES HP12x53 WITH MINIMUM PILE EMBEDMENT INTO PILE CAP OF 1'-0"
 - SEE TYPICAL END POST DETAIL ON SHEET #16

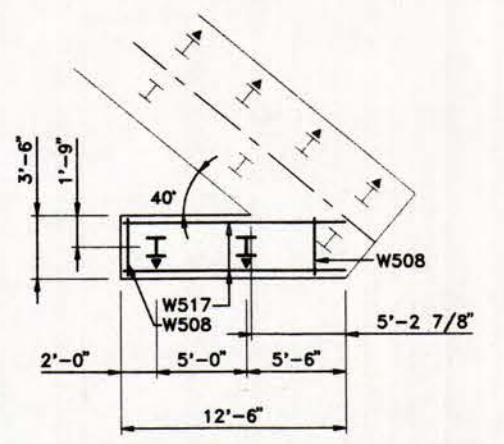
▼ DENOTES PILES BATTERED @4:12



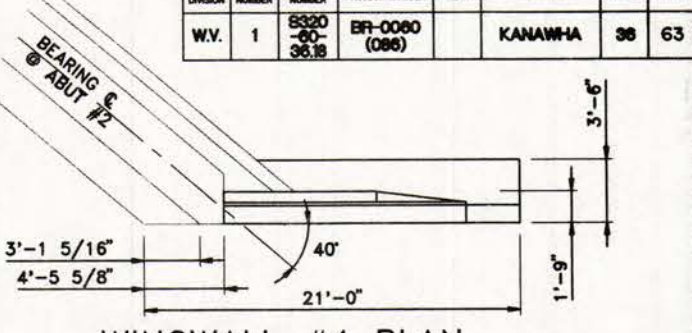
WINGWALL #3 PLAN
SCALE: 3/16" = 1'-0"



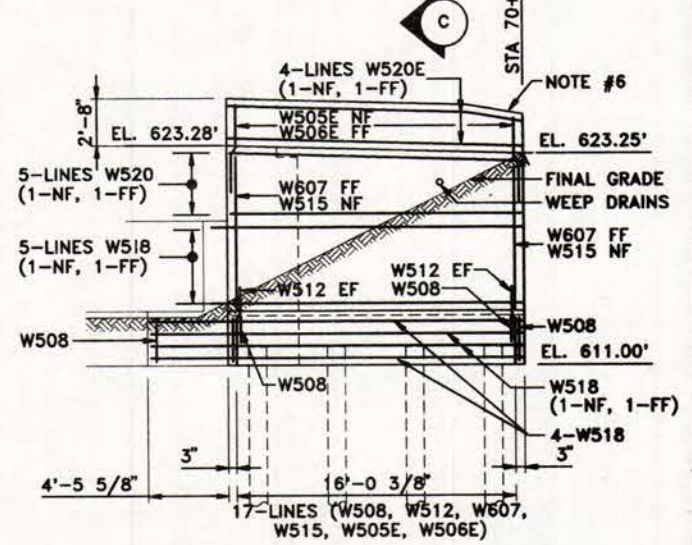
WINGWALL #3 ELEVATION
SCALE: 3/16" = 1'-0" (PHASE III CONST.)



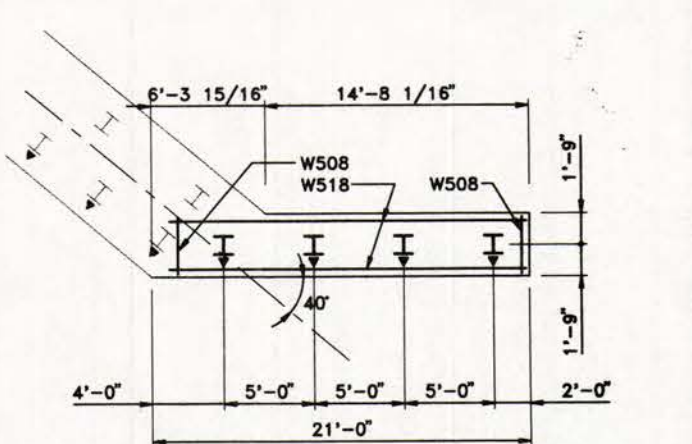
WINGWALL #3 FOOTING PLAN
SCALE: 3/16" = 1'-0"



WINGWALL #4 PLAN
SCALE: 3/16" = 1'-0"



WINGWALL #4 ELEVATION
SCALE: 3/16" = 1'-0" (PHASE II CONSTRUCTION)



WINGWALL #4 FOOTING PLAN
SCALE: 3/16" = 1'-0"

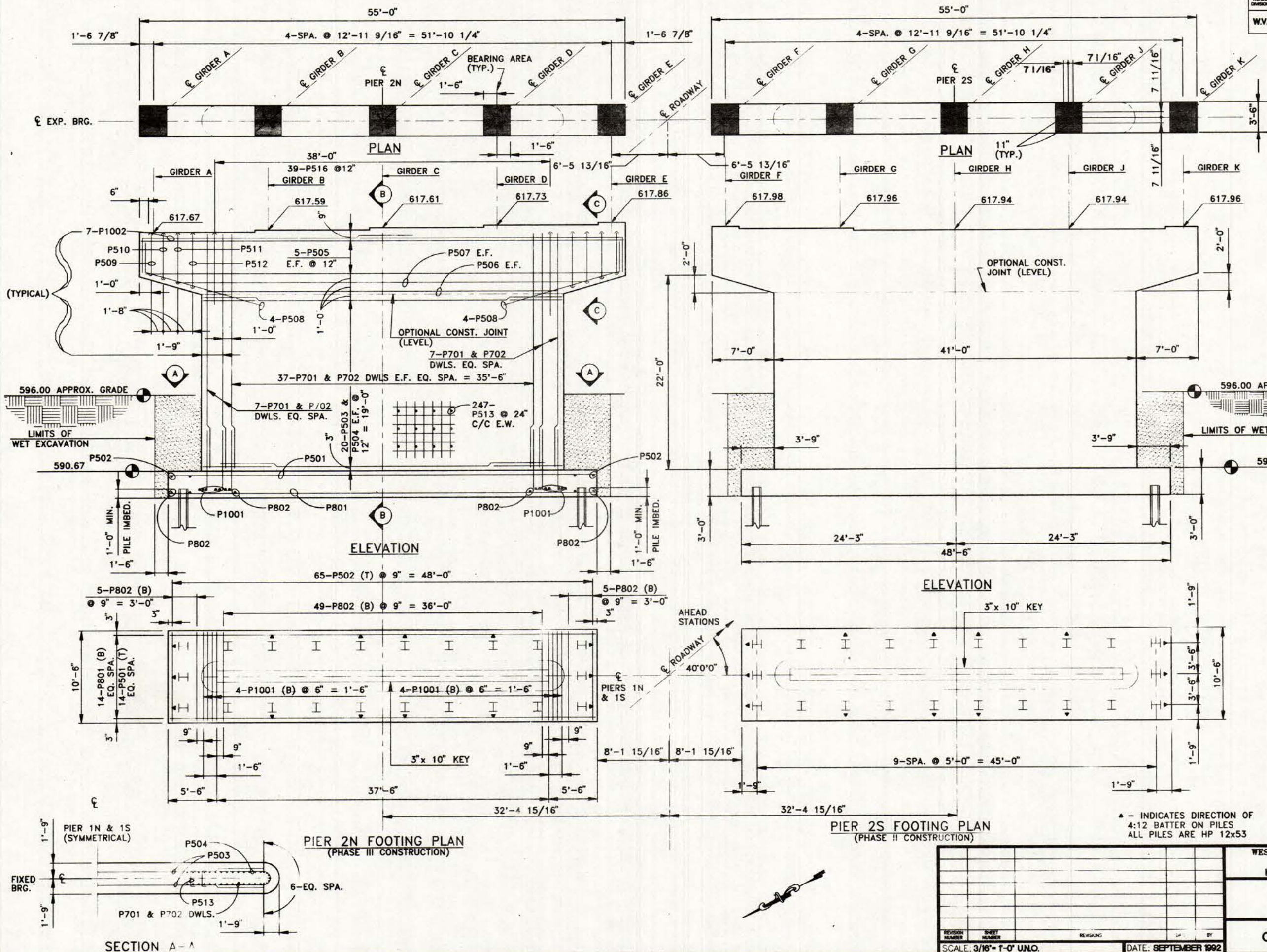
REVISION NUMBER	DESCRIPTION	DATE	BY
36	CORRECT NOTE	3-23-94	JED

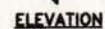
WEST VIRGINIA DEPARTMENT OF TRANSPORTATION
DIVISION OF HIGHWAYS
KELLY CREEK BRIDGE #3886 REPLACEMENT

WINGWALLS 2, 3, + 4

Chapman Technical Group

PUBLIC ROADS DIVISION	STATE DISTRICT NUMBER	STATE PROJECT NUMBER	FEDERAL PROJECT NUMBER	FISCAL YEAR	COUNTY	SHEET NO.	TOTAL SHEETS
W.V.	1	8320-60-36.8	BR-0080 (086)		KANAWHA	28	63



[illegible]

1'-0" GIRDER A ONLY

637 11, 400

1000

637 11, 400

1'-7" 60'-0" 80'-0" 60'-0" 1'-7"

5" 8-SPA @ 5" = 3'-4" 16'-3" 16'-0" 48'-0" 16'-0" 16'-3" 5"

48-SPA @ 10" = 40'-0" 14'-7" 40-SPA @ 1'-1" = 43'-4" 14'-7" 48-SPA @ 10" = 40'-0"

3'-9" 3'-9"

SHEAR STUD (TYP.) SEE DETAIL THIS SHEET

W36x150 M270 GRADE 50

W36x135 M270 GRADE 50

W36x150 M270 GRADE 50

EXP. BRG. ABUT. #1

FIXED BRG. PIER #1

EXP. BRG. PIER #2

EXP. BRG. ABUT. #2

SEE SPLICE DETAIL THIS SHEET

♀ SPLICE

(CHANGES IN SLOPE OF GIRDERS OCCUR @ SPLICES)

9000

329

297

336 000

GIRDER ELEVATION (TYP. FOR (A) THRU (K))

NOTES:

GIRDERS SHALL BE FABRICATED SO THAT ENDS, BEARING STIFFENERS, AND CONNECTION PLATES AT END DIAPHRAGMS ARE PLUMB UNDER FULL DEAD LOAD.

CAMBER IS NOT REQUIRED. TURN NATURAL CAMBER UP.
ALL DIAPHRAGMS ARE TO BE MC 18x42.7 & M270 GRADE
36 STEEL.

ALL GIRDERS ARE TO BE M270 GRADE 50 STEEL.

ALL SPLICE PLATES ARE TO BE M270 GRADE 50 STEEL.

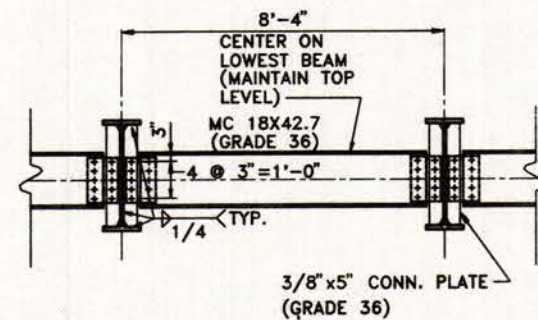
ALL LONGITUDINAL AND TRANSVERSE DIMENSIONS ARE MEASURED HORIZONTALLY.

OPEN HOLES SHALL BE 15/16" UNLESS NOTED. FIELD CONNECTIONS SHALL BE MADE WITH 7/8" HIGH STRENGTH BOLTS

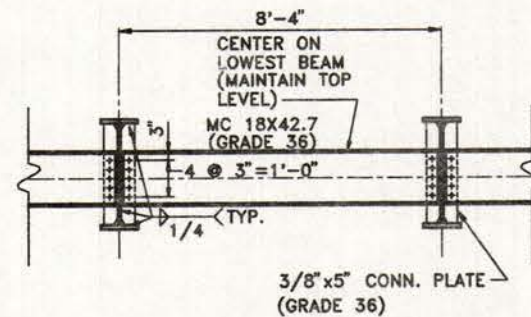
WEST VIRGINIA DEPARTMENT OF TRANSPORTATION
DIVISION OF HIGHWAYS
KELLY CREEK BRIDGE #3886 REPLACEMENT

FRAMING PLAN

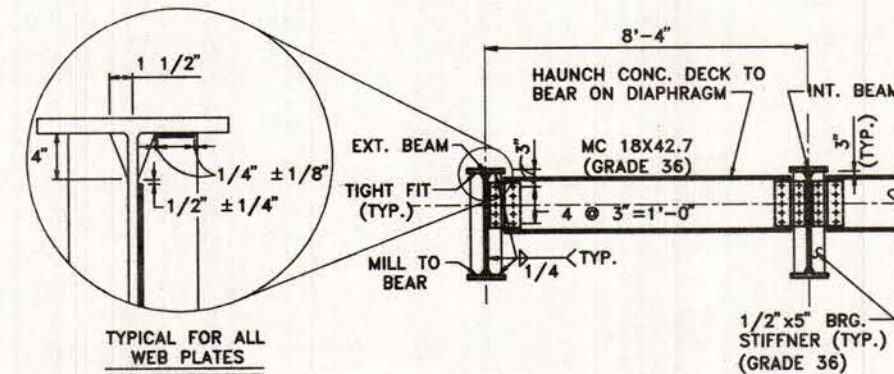
Chapman Technical Group 



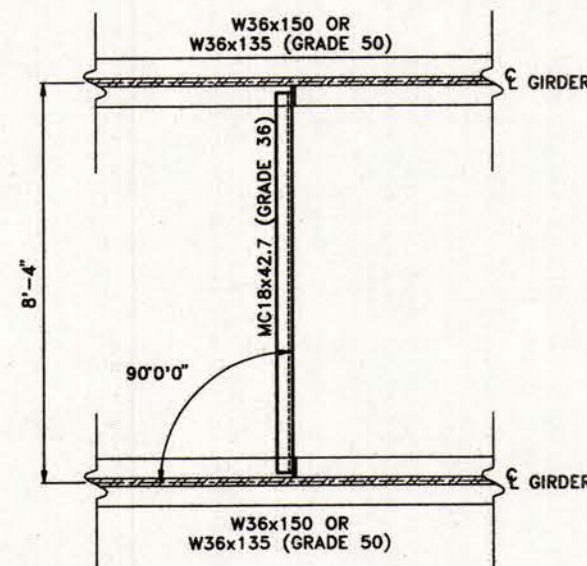
PIER DIAPHRAGM
NO SCALE



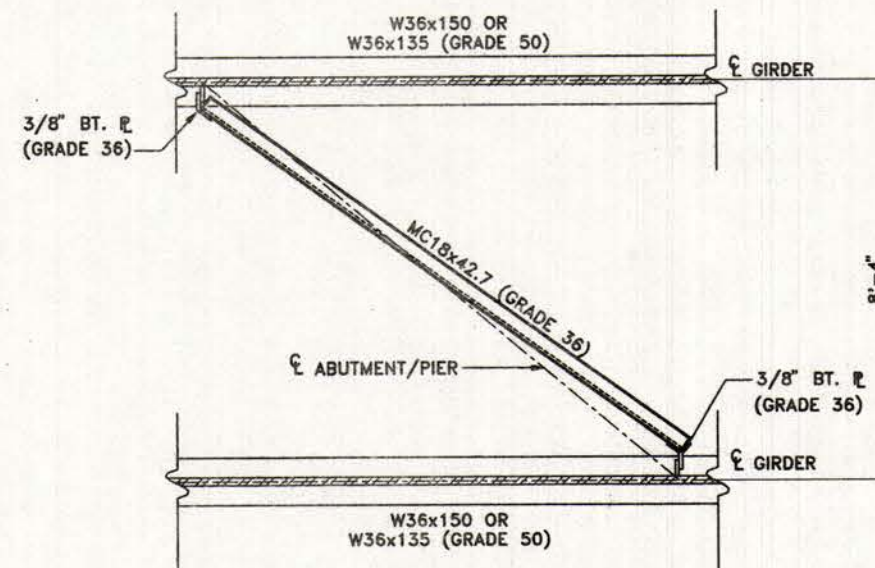
INTERMEDIATE DIAPHRAGM
NO SCALE



ABUTMENT DIAPHRAGM
NO SCALE

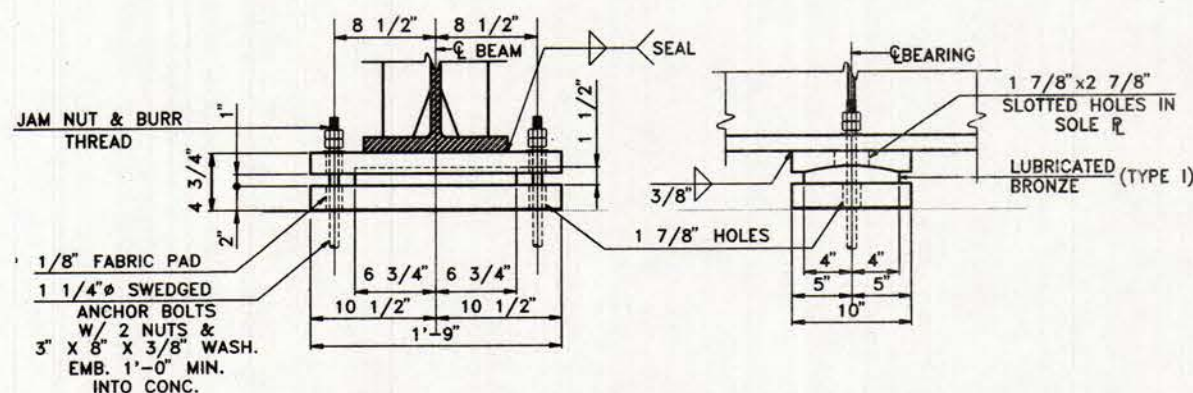


INTERMEDIATE DIAPHRAGM PLAN
NO SCALE



ABUTMENT AND PIER DIAPHRAGM PLAN
NO SCALE

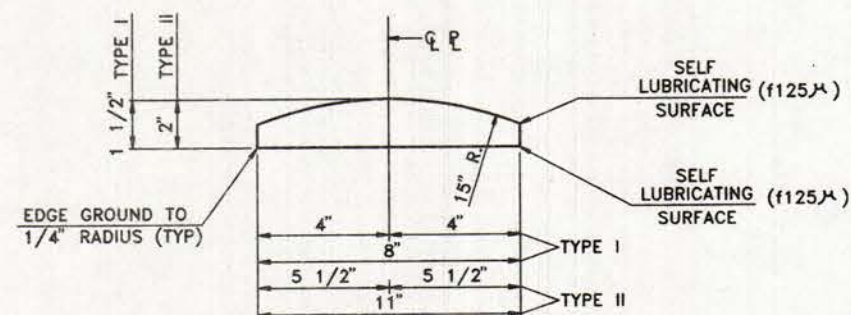
REVISION NUMBER	SHEET NUMBER	REVISIONS	DATE	BY



FOR ALL BEAMS (20 REQ'D.)

ABUTMENT 1 & 2 EXPANSION BEARING

SCALE: 1 1/2" = 1'-0"

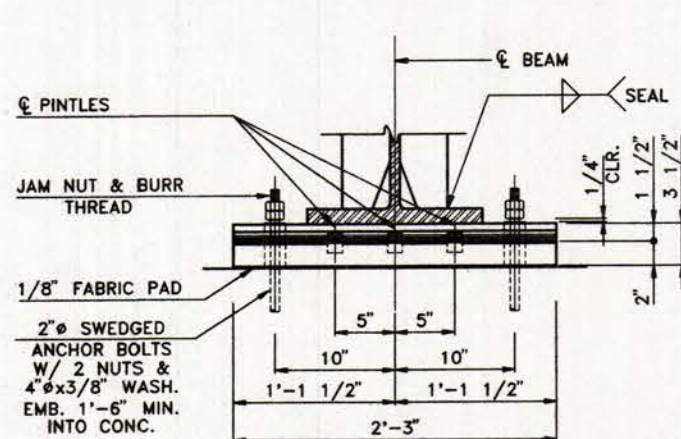


BRONZE PLATE (PIER #2) TYPE II

SCALE: NONE

BRONZE PLATE (ABUT. #1 & #2) TYPE I

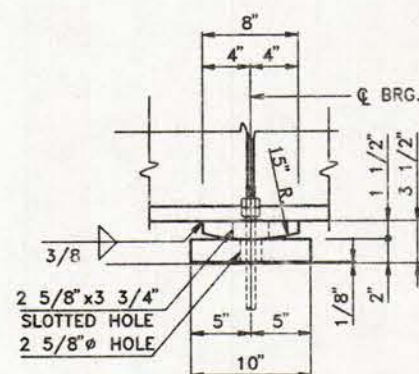
SCALE: NONE



FOR ALL BEAMS (10 REQ'D.)

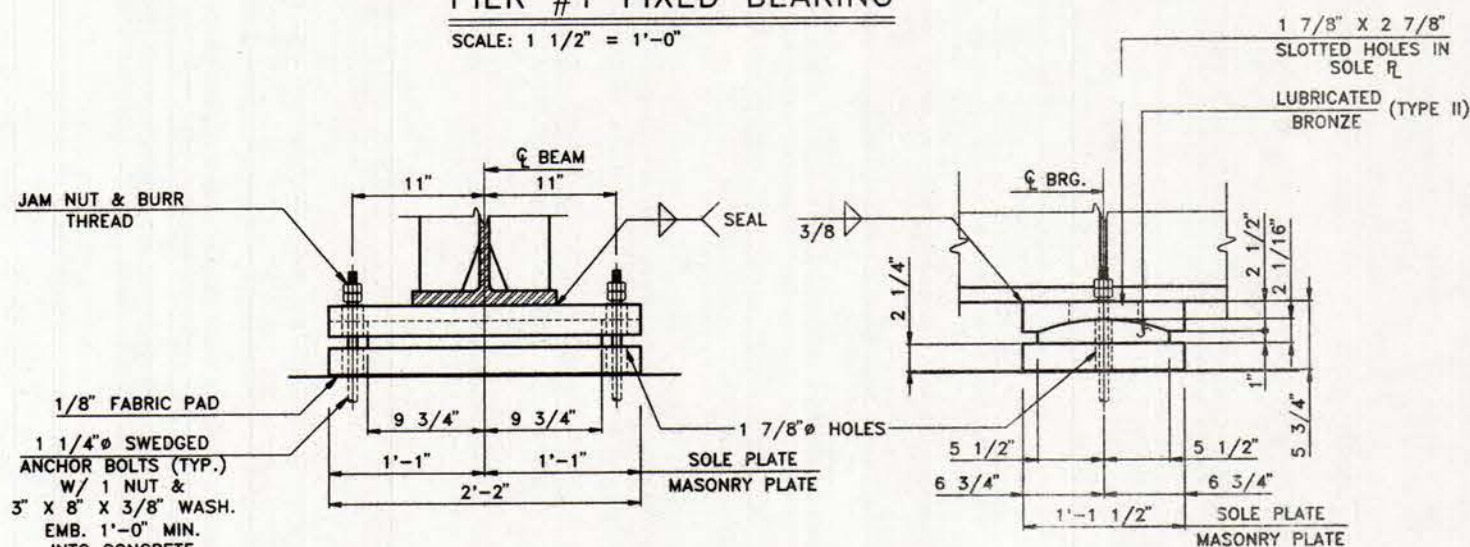
PIER #1 FIXED BEARING

SCALE: 1 1/2" = 1'-0"



PINTLE DETAIL

SCALE: 1/2" = 1"



FOR ALL BEAMS (10 REQ'D.)

PIER #2 EXPANSION BEARING

SCALE: 1 1/2" = 1'-0"

BEARING NOTES

FILL SLOTS AND HOLES IN MASONRY PLATE AROUND ANCHOR BOLTS WITH APPROVED NON-HARDENING CAULKING COMPOUND OR ELASTIC JOINT SEALER.

STEEL SURFACES OF SOLE PLATE, ROCKER PLATE, WEB AND BEARING PLATE TO BE A MACHINE FINISH AS SHOWN ON THE DETAILS, MEASURED IN ACCORDANCE WITH ANSI B46.1. OTHER STEEL PLATE SURFACES TO BE AT LEAST 1000 INCH.

BEARINGS SHALL BE SHOP ASSEMBLED AND MATCH MARKED TO INSURE PROPER FIT.

PLATE THICKNESS SHOWN ARE AT THE CENTERLINES OF BEARINGS AT 70°F.

SOLE PLATE TO BE LEVELED OR HAVE RADIUS MACHINED TO MATCH GRADE WHEN GRADE EXCEEDS 3% FOR FIXED BEARING OR 1% FOR ALL OTHER BEARINGS.

STEEL PLATES SHALL MEET A FLATNESS REQUIREMENT OF 0.005 INCH/INCH MAXIMUM IN DIRECTION BEING MEASURED (WIDTH, LENGTH AND DIAGONALS) BUT NOT TO EXCEED 1/8 INCH.

BEARING SURFACES SHALL BE FINISHED IN ACCORDANCE WITH SECTION 615.23, FINISH IN THE DIRECTION OF ROTATION.

ALL SHOE STEEL, INCLUDING SOLE AND MASONRY PLATES AND ANCHOR BOLTS SHALL BE PAINTED WITH 4 MILS OF ZINC RICH PAINT IN ACCORDANCE WITH SECTION 711.20, THE FIELD COAT SHALL BE THE SAME AS FOR STRUCTURAL STEEL.

A FABRIC PAD FOR LEVELING SHALL BE PLACED UNDER THE MASONRY PLATE. THE COST OF THE BRONZE PLATES AND FABRIC PADS SHALL BE INCLUDED IN ITEM 615-01.

ALL SOLE PLATES AND MASONRY PLATES ARE TO BE AASHTO M270 G236.

EXPANSION BEARING NOTES

SURFACES OF SOLE PLATE AND MASONRY PLATE IN CONTACT WITH BRONZE PLATE TO HAVE A MACHINE FINISH OF AT LEAST 125 INCH. THESE STEEL SURFACES SHALL NOT BE PAINTED, BUT SHALL BE COATED WITH A MULTIPURPOSE GREASE BEFORE SHIPMENT. COATING SHALL BE REMOVED WITH A SOLVENT IMMEDIATELY PRIOR TO ERECTION.

SELF-LUBRICATING BRONZE BEARING PLATES SHALL CONFORM TO THE REQUIREMENTS OF ASTM B22, COPPER ALLOY UNS C91100 MODIFIED WITH UP TO 2.5% LEAD MAXIMUM. THE SLIDING SURFACES OF THE PLATES SHALL BE PROVIDED WITH ANNULAR GROOVES OR CYLINDRICAL RECESSES, OR A COMBINATION THEREOF, WHICH SHALL BE FILLED WITH A LUBRICATING COMPOUND. THE LUBRICATING COMPOUND SHALL BE COMPRESSED INTO THE RECESSES UNDER SUFFICIENT PRESSURE TO FORM A NON-PLASTIC LUBRICATING INSET, THE LUBRICATING INSET SHALL COMPRISE NOT LESS THAN 25% OF THE TOTAL AREA OF THE PLATE. THE FRICTIONAL COEFFICIENT SHALL NOT EXCEED 0.10R, WHEN BEARING IS TESTED UNDER A LOAD OF R AND FOR 1000 CYCLES.

THE COMPOUND SHALL BE FREE OF ANY MATERIAL WHICH COULD CAUSE ABRASIVE OR CORROSIVE ACTION UPON THE METAL SURFACES AND ALSO SHALL BE ABLE TO WITHSTAND EXTREMELY HIGH PRESSURES AND ATMOSPHERIC ELEMENTS OVER LONG PERIODS OF TIME.

ALL ITEMS SHALL BE THE STANDARD PRODUCTS OF THE MANUFACTURER OF SUCH MATERIALS FOR THIS APPLICATION.

PRIOR TO ASSEMBLY IN PLACE, THE STEEL SURFACES WHICH WILL BEAR ON THE SELF-LUBRICATING BEARING PLATE SHALL BE THOROUGHLY LUBRICATED WITH ADDITIONAL ANTI-OXIDANT LUBRICANT FURNISHED BY THE MANUFACTURER. THESE STEEL SURFACES SHALL NOT BE PAINTED.

WEST VIRGINIA DEPARTMENT OF TRANSPORTATION DIVISION OF HIGHWAYS KELLY CREEK BRIDGE #3886 REPLACEMENT			
BEARING DETAILS			
Chapman Technical Group			
REVISION NUMBER	SHEET NUMBER	REVISIONS	DATE BY
SCALE: AS SHOWN		DATE: SEPTEMBER 1992	

[illegible]

Diagram illustrating the plan view of Bridge Deck A, showing the deck overhang measurements at 1/10 points along Span 1 of Girder A. The measurements are as follows:

- 2'-3 5/32"
- 2'-5 19/32"
- 2'-7 25/32"
- 2'-9 21/32"
- 2'-11 1/4"
- 3'-0 9/16"
- 3'-1 19/32"
- 3'-2 11/32"
- 3'-2 13/16"
- 3'-5"
- 3'-5"

Other labels include: BEARING ABUT #1, GIRDER A, BEARING PIER #1, BRIDGE DECK FACIA, PT STA 68+14.04, and 6'-0" TYPICAL.

(DECK OVERHANG MEASURED AT 1/10 POINTS ALONG SPAN 1 OF GIRDER A)

QUANTITIES		
ITEM	UNIT	QUANTITY
CLASS K CONCRETE	C.Y.	447 (46)
EPOXY COATED REINFORCING STEEL BARS	LBS	164,510

REVISION NUMBER

42
SHEET NUMBER

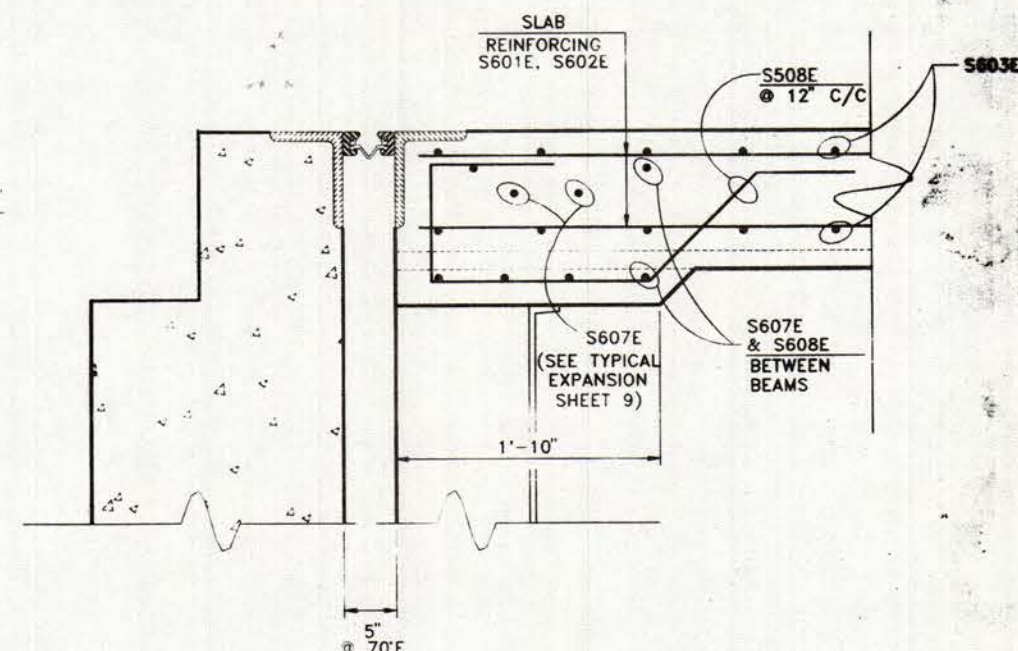
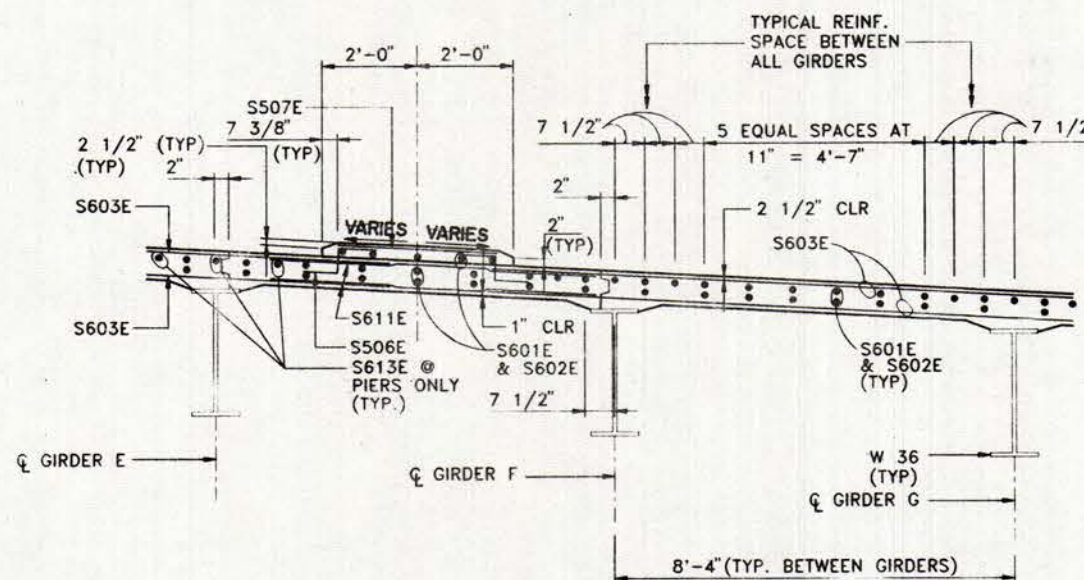
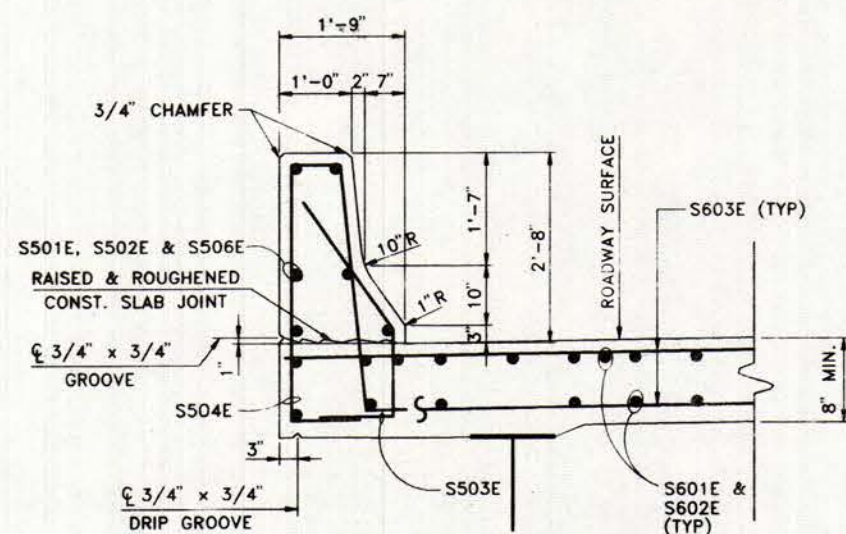
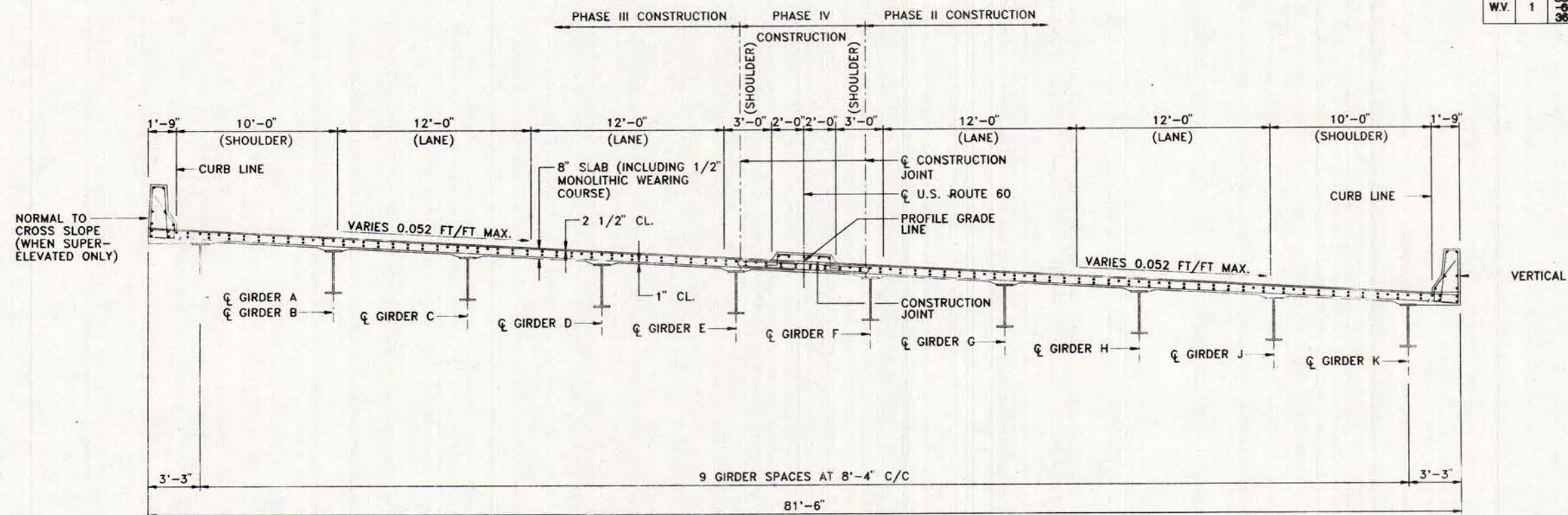
CORRECT QTY. FOR CLASS K CONC.
REVISIONS

9-17-98
DATE BY JEL

SCALE: $\frac{3}{32}'' = 1'-0''$ DATE: SEPTEMBER 1998

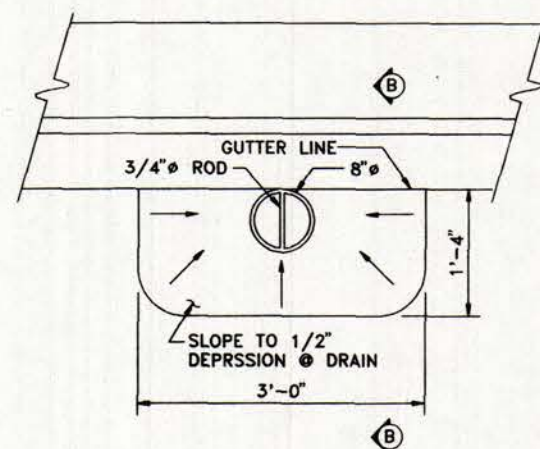
Chapman Technical Group 

PUBLIC ROADS DIVISION	STATE DISTRICT NUMBER	STATE PROJECT NUMBER	FEDERAL PROJECT NUMBER	FISCAL YEAR	COUNTY	SHEET NUMBER	TOTAL SHEETS
W.V.	1	8320-60-36.18	BR-0060 (086)		KANAWHA	48	63



REVISION NUMBER	SHEET NUMBER	REVISIONS
1	48	
2	49	
3	50	
4	51	
5	52	
6	53	
7	54	
8	55	
9	56	
10	57	
11	58	
12	59	
13	60	
14	61	
15	62	
16	63	

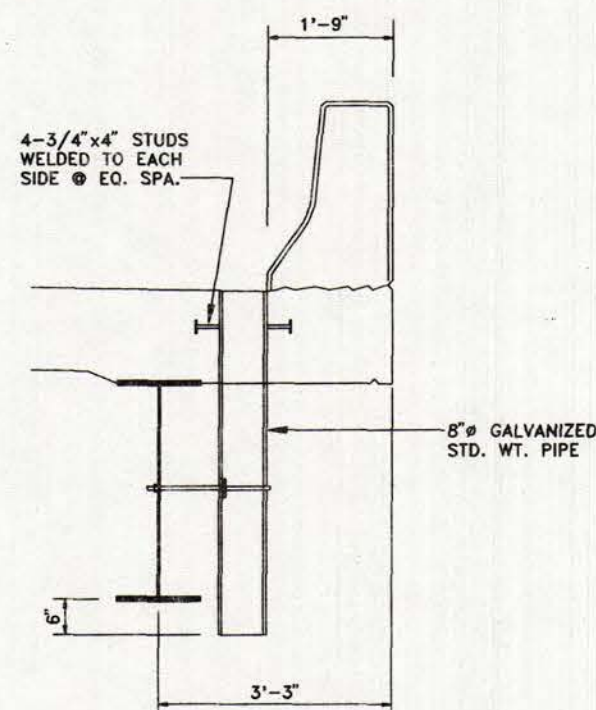
WEST VIRGINIA DEPARTMENT OF TRANSPORTATION DIVISION OF HIGHWAYS KELLY CREEK BRIDGE #3886 REPLACEMENT
DECK DETAILS
Chapman Technical Group
DATE: SEPTEMBER 1992



DECK DRAIN DETAIL

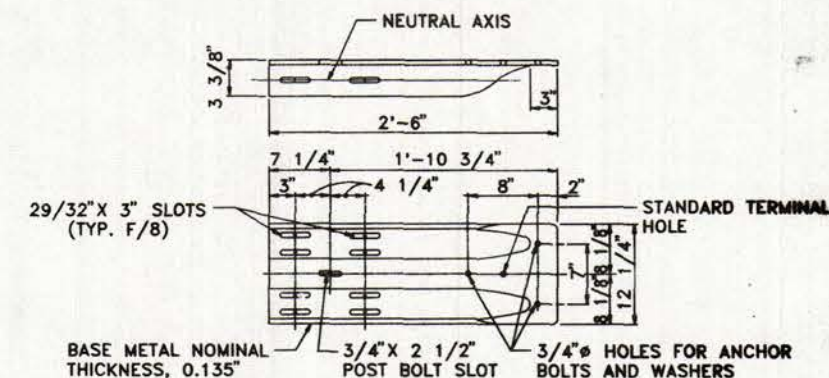
SCALE: 1" = 1'-0"

(GALVANIZE ASSEMBLY ASTM AASHTO M111)
(INCIDENTAL TO CLASS "K" CONCRETE)



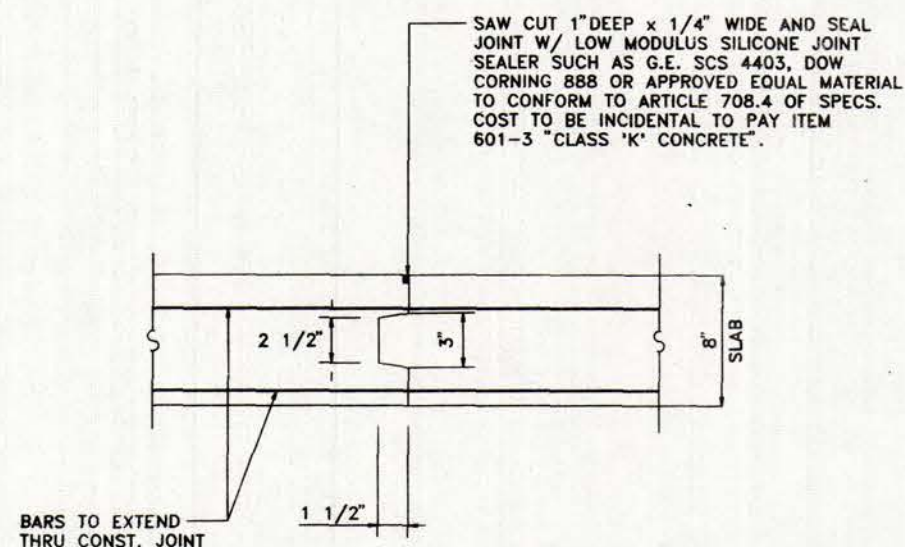
SECTION B-B

SCALE: 3/4" = 1'-0"



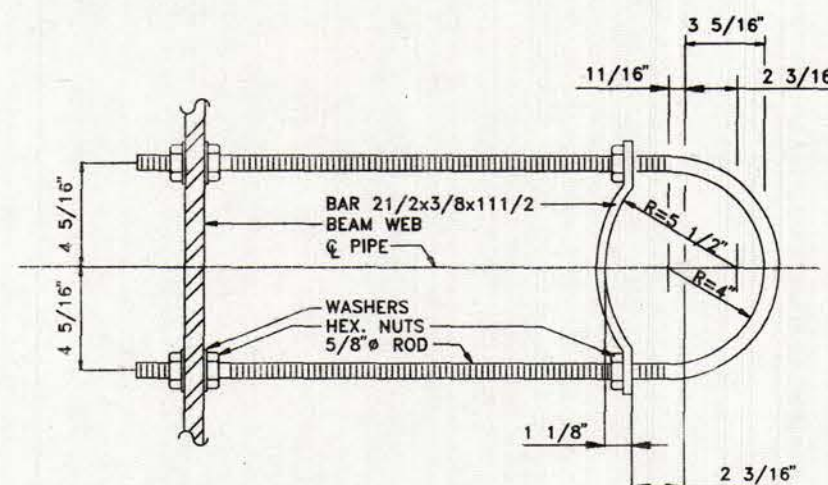
GUARDRAIL END SHOE

NO SCALE



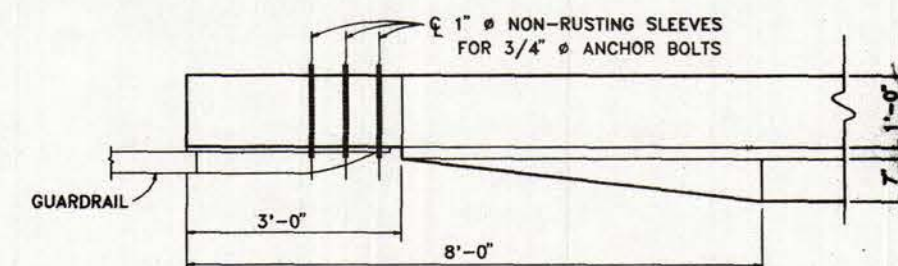
CONSTRUCTION JOINT DETAIL

SCALE: NONE



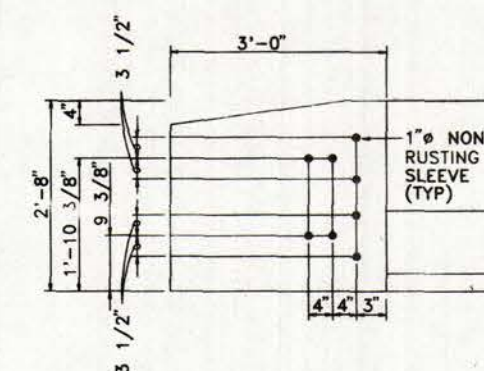
DECK DRAIN PIPE CLAMP

SCALE: 3" = 1'-0"



END POST PLAN

SCALE: 3/4" = 1'-0"



TYP. END POST ELEVATION

SCALE: 3/4" = 1'-0"

NOTES:

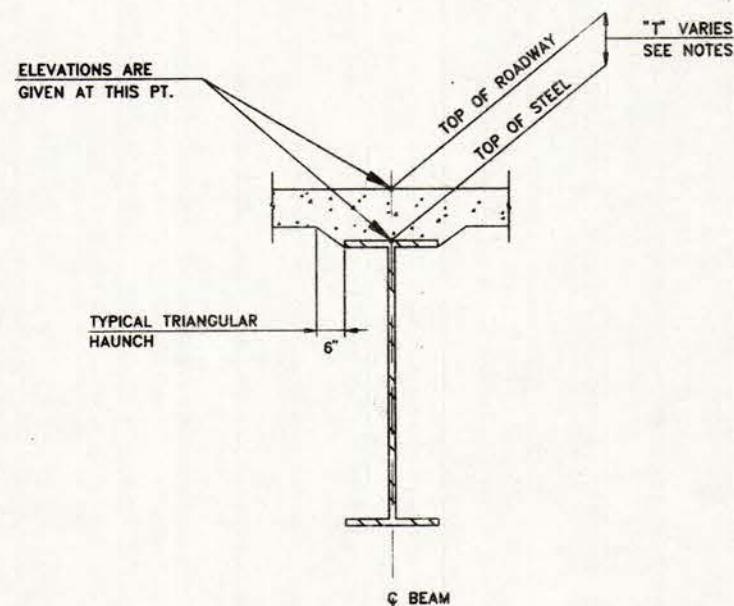
7/8" ANCHOR BOLTS WITH 3" X 3" X 1/4" WASHER AND 2 HEX. NUTS. ANCHOR BOLTS TO BE ENCASED IN 1" I.D. SLEEVE OF NON RUSTING MATERIAL. GALVANIZE ANCHOR BOLTS IN ACCORDANCE WITH A.S.T.M. A123-203

CONCRETE, SLEEVES & BOLTS FOR ENDPOSTS ARE INCLUDED IN ITEM 601-02 CLASS B CONCRETE

DECK DRAIN NOTES

- WHERE DECK DRAINS INTERFERE WITH REINFORCING BARS IN DECK SLAB, THE BARS SHALL BE CUT AS DIRECTED BY THE ENGINEER.
- PAYMENT FOR DECK DRAINS, SUPPORTS AND PIPES SHALL BE MADE UNDER ITEM 615-01, STEEL SUPERSTRUCTURE.
- ALL PIPE AND FITTINGS SHALL BE 8" DIAMETER STANDARD BLACK STEEL PIPE IN ACCORDANCE WITH SECTION 709.17.
- ALL EXPOSED SURFACES OF PIPES, FRAMES AND GRATINGS SHALL BE PAINTED AS PER GENERAL NOTES EXCEPT THAT THE INSIDE SURFACES OF PIPE SHALL RECEIVE TWO(2) COATS OF SHOP PAINT. HOT DIP GALVANIZING IN ACCORDANCE WITH ASTM A123, TWO OUNCES PER SQUARE FOOT, MAY BE USED IN LIEU OF PAINTING.

	SPAN 1										SPAN 2										SPAN 3										E BEARING	
	E BEARING	TOP OF SLAB ELEVATIONS									E BEARING	TOP OF SLAB ELEVATIONS									E BEARING	TOP OF SLAB ELEVATIONS										
		ABUT 1	.10	.20	.30	.40	.50	.60	.70	.80		.90	PIER 1	.10	.20	.30	.40	.50	.60	.70		.80	.90	PIER 2	.10	.20	.30	.40	.50	.60		.70
GIRDER A	624.08	623.94	623.81	623.67	623.55	623.43	623.31	623.20	623.10	622.99	622.89	622.77	622.65	622.54	622.43	622.33	622.24	622.16	622.08	622.01	621.95	621.91	621.87	621.84	621.81	621.78	621.76	621.74	621.75	621.78	621.82	
B	623.50	623.39	623.27	623.17	623.07	622.97	622.88	622.79	622.70	622.62	622.54	622.45	622.36	622.28	622.20	622.14	622.08	622.02	621.98	621.94	621.90	621.89	621.87	621.86	621.85	621.85	621.87	621.88	621.93	622.00	622.07	
C	622.99	622.90	622.81	622.72	622.65	622.57	622.50	622.43	622.37	622.31	622.26	622.20	622.13	622.08	622.04	622.00	621.97	621.95	621.93	621.93	621.92	621.93	621.93	621.94	621.96	621.98	622.04	622.10	622.17	622.25	622.33	
D	622.54	622.47	622.40	622.34	622.29	622.23	622.19	622.14	622.10	622.07	622.04	622.00	621.97	621.95	621.94	621.93	621.93	621.94	621.98	621.98	622.00	622.03	622.06	622.10	622.15	622.21	622.28	622.35	622.43	622.51	622.60	
E	622.15	622.10	622.06	622.02	621.99	621.96	621.93	621.92	621.90	621.89	621.88	621.87	621.88	621.89	621.90	621.93	621.96	621.99	622.09	622.09	622.15	622.20	622.25	622.31	622.38	622.46	622.53	622.61	622.70	622.79	622.88	
F	621.83	621.80	621.78	621.77	621.76	621.76	621.75	621.75	621.76	621.76	621.77	621.78	621.80	621.83	621.87	621.91	621.96	622.02	622.15	622.15	622.23	622.29	622.36	622.43	622.51	622.58	621.67	622.76	622.85	622.94	623.04	
G	621.57	621.57	621.58	621.59	621.60	621.61	621.62	621.63	621.63	621.64	621.65	621.68	621.71	621.75	621.79	621.84	621.90	621.97	622.12	622.12	622.21	622.28	622.35	622.43	622.51	622.60	622.69	622.78	622.88	622.98	623.08	
H	621.38	621.40	621.43	621.45	621.48	621.50	621.50	621.51	621.52	621.54	621.55	621.59	621.63	621.67	621.73	621.79	621.85	621.93	622.10	622.10	622.19	622.27	622.35	622.44	622.53	622.62	622.72	622.81	622.92	623.03	623.14	
J	621.25	621.30	621.34	621.37	621.37	621.38	621.39	621.40	621.42	621.44	621.46	621.51	621.55	621.61	621.67	621.74	621.82	621.90	622.09	622.09	622.19	622.28	622.36	622.46	622.55	622.65	622.75	622.86	622.97	623.09	623.20	
K	621.19	621.21	621.23	621.24	621.25	621.36	621.28	621.30	621.33	621.36	621.39	621.44	621.49	621.56	621.63	621.71	621.79	621.88	622.09	622.09	622.20	622.29	622.39	622.48	622.59	622.69	622.80	622.92	623.05	623.16	623.29	
A-D	-----	0.01	0.02	0.02	0.03	0.02	0.02	0.01	0.01	-----	-----	0.01	0.03	0.05	0.06	0.07	0.06	0.05	0.03	0.01	-----	-----	0.01	0.01	0.02	0.02	0.03	0.02	0.01	623.03	-----	
E-F	-----	0.01	0.01	0.01	0.02	0.01	0.01	0.01	-----	-----	-----	0.01	0.02	0.03	0.04	0.04	0.04	0.03	0.02	0.01	-----	-----	-----	0.01	0.01	0.01	0.02	0.01	0.01	623.09	-----	
G-K	-----	0.01	0.02	0.02	0.03	0.02	0.02	0.01	0.01	-----	-----	0.01	0.02	0.05	0.06	0.07	0.06	0.05	0.03	0.01	-----	-----	0.01	0.01	0.02	0.02	0.03	0.02	0.01	623.16	-----	



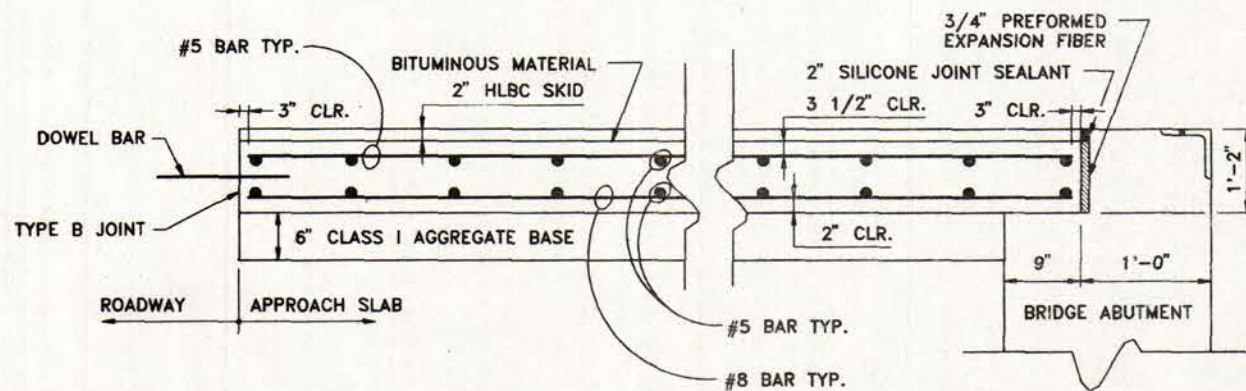
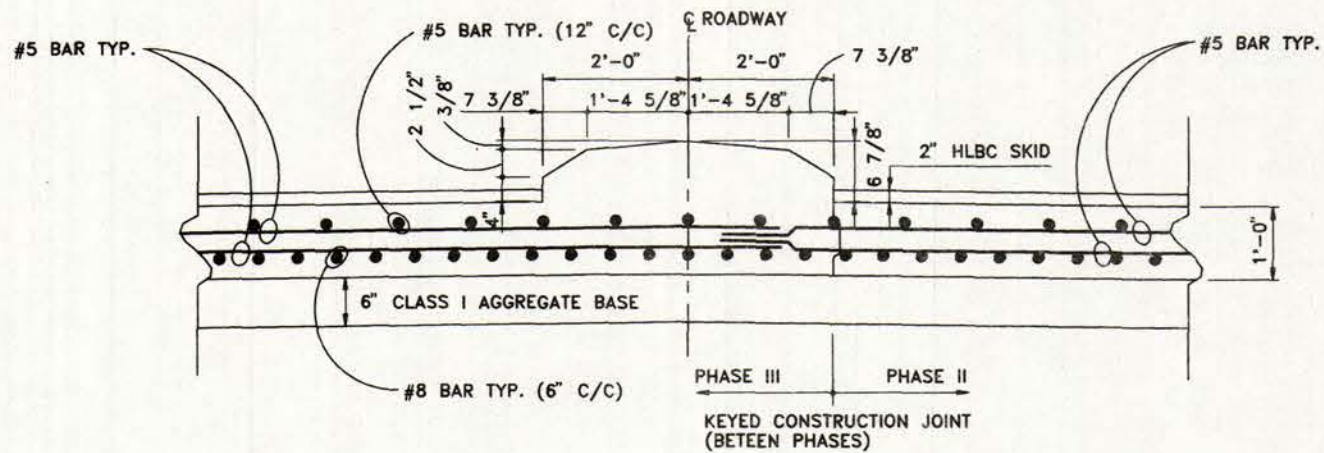
HAUNCH DETAIL
NO SCALE

NOTES

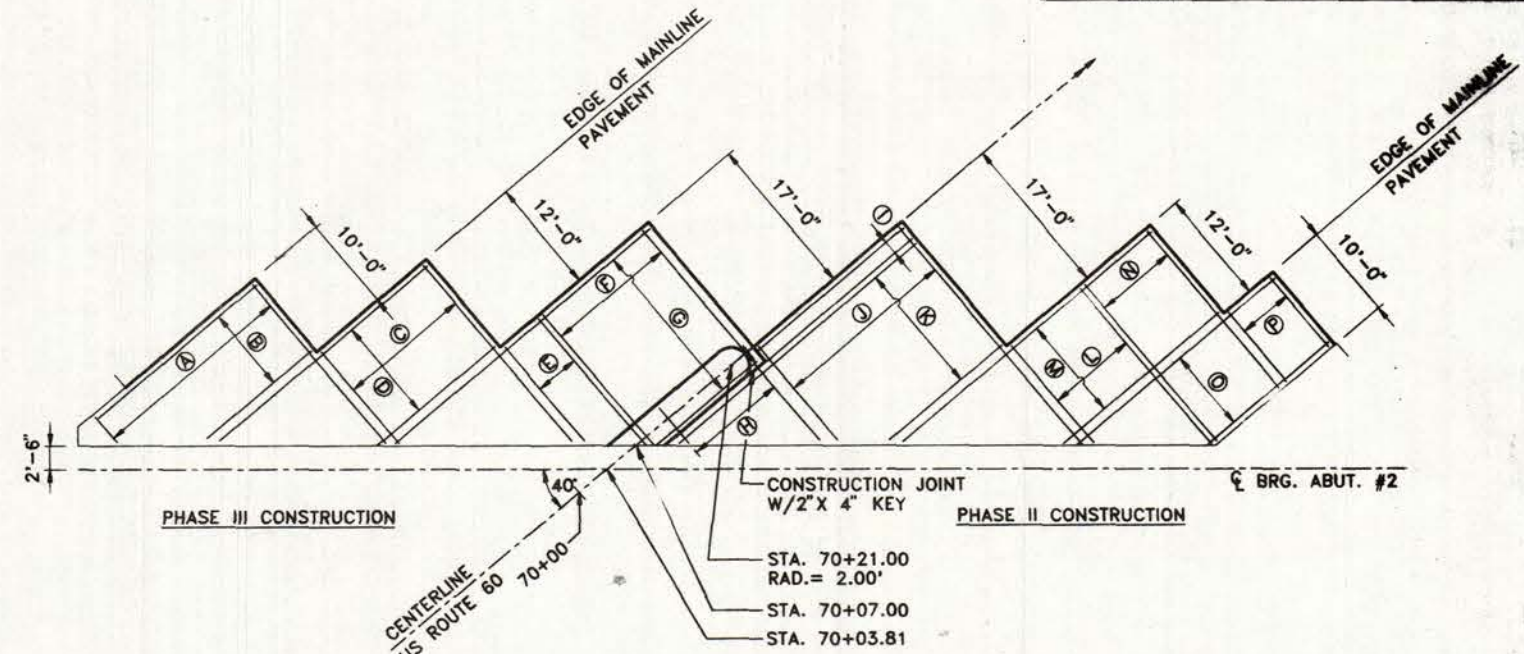
- TABULATED DEFLECTIONS ARE DUE TO THE CONCRETE SLAB ONLY. DEFLECTIONS FROM BARRIERS, MEDIAN & FUTURE WEARING SURFACE ARE NOT INCLUDED. FOR GIRDERS E & F THE DEFLECTIONS ARE REDUCED FOR PHASE II AND PHASE III CONSTRUCTION ONLY. THE DEFLECTIONS CAUSED BY PHASE IV CONSTRUCTION ARE NEGLIGIBLE SINCE THE LOAD WILL BE DISTRIBUTED OVER ALL 10 GIRDERS.
 - ELEVATIONS OF THE TOP OF ROADWAY ARE AT CENTERLINE OF BEAM. AFTER ALL BEAMS HAVE BEEN ASSEMBLED AND FALSEWORK REMOVED AND BEFORE ANY CONCRETE DECK IS POURED, THE ENGINEER SHALL TAKE ELEVATIONS ALONG THE TOP OF STEEL AT POINTS WHERE GRADE ELEVATIONS ARE GIVEN. THE DIFFERENCE BETWEEN THIS ELEVATION AND THE GRADE ELEVATION SHOWN, PLUS THE AMOUNT THE DEFLECTION SHOWN AT EACH POINT DUE TO THE D.L. OF CONCRETE, WILL BE THICKNESS "T" AT THESE POINTS (SEE HAUNCH DETAIL).
- THE MINIMUM THICKNESS "T" SHALL NOT BE LESS THEN 8" AT THE CENTERLINE OF BEAMS. IF THE BEAMS HAVE EXCESSIVE CAMBER AND THIS MINIMUM CANNOT BE OBTAINED, THE GRADE SHALL BE ADJUSTED TO OBTAIN THIS THICKNESS.

WEST VIRGINIA DEPARTMENT OF TRANSPORTATION DIVISION OF HIGHWAYS KELLY CREEK BRIDGE #3886 REPLACEMENT			
DECK SLAB ELEVATIONS AND DEFLECTION TABLE			
Chapman Technical Group			
REVISION NUMBER	SHEET NUMBER	DATE	BY
SCALE: NONE	DATE: SEPTEMBER 1992		

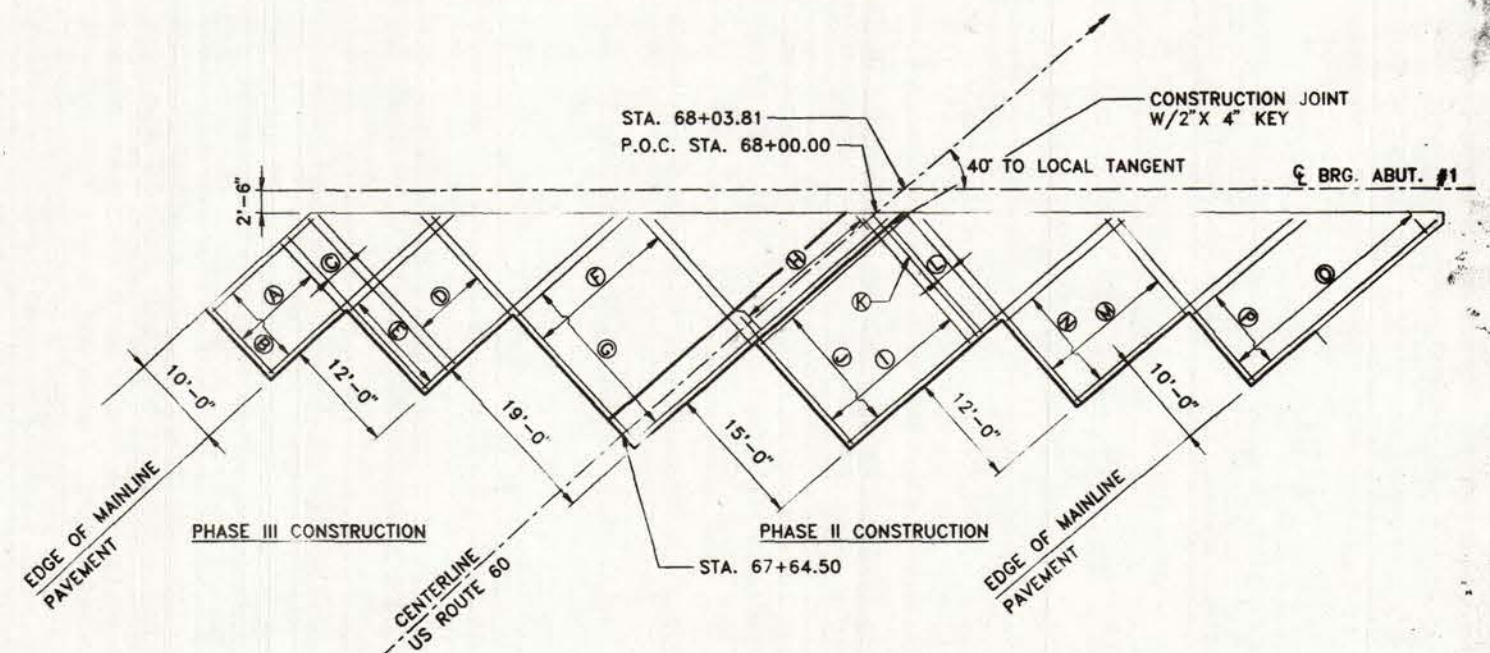
PROFILE	DATE	BY	LAN	DATE	BY
SURVEYED	✓		SURVEYED	✓	
NOTE BOOK	✓		NOTE BOOK	✓	
NO.	✓		NO.	✓	
GRADES CHECKED	✓		GRADES CHECKED	✓	
S.M. 1/4 NOTED	✓		S.M. 1/4 NOTED	✓	
STRICTLY MOTIVATING CTRD.	✓		STRICTLY MOTIVATING CTRD.	✓	



APPROACH SLAB #1		APPROACH SLAB #2	
NO.	BAR DESCRIPTION	NO.	BAR DESCRIPTION
A	20- AS501E (T&B) @ 12" c/c	A	48- AS517E (T&B) @ 12" c/c
B	10- AS502E (T) @ 12" c/c	B	10- AS518E (T) @ 12" c/c
	20- AS801E (B) @ 6" c/c		20- AS807E (B) @ 6" c/c
C	8- AS503E (T&B) @ 12" c/c	C	30- AS519E (T&B) @ 12" c/c
D	18- AS504E (T&B) @ 12" c/c	D	12- AS520E (T) @ 12" c/c
E	12- AS505E (T) @ 12" c/c		24- AS808E (B) @ 6" c/c
	24- AS802E (B) @ 6" c/c	E	12- AS521E (T&B) @ 12" c/c
F	32- AS506E (T&B) @ 12" c/c	F	28- AS522E (T&B) @ 12" c/c
G	19- AS507E (T) @ 12" c/c	G	19- AS523E (T) @ 12" c/c
	38- AS803E (B) @ 6" c/c		38- AS809E (B) @ 6" c/c
H	34- AS508E (T&B) @ 12" c/c	H	24- AS524E (T&B) @ 12" c/c
I	34- AS509E (T&B) @ 12" c/c	I	2- AS525E (T) @ 12" c/c
J	15- AS510E (T) @ 12" c/c		4- AS810E (B) @ 6" c/c
	30- AS804E (B) @ 6" c/c	J	20- AS526E (B) @ 12" c/c
K	2- AS511E (T&B)	K	15- AS527E (T) @ 12" c/c
L	6- AS512E (T&B) @ 12" c/c		30- AS811E (B) @ 6" c/c
M	32- AS513E (T&B) @ 12" c/c	L	20- AS528E (T&B) @ 12" c/c
N	12- AS514E (T) @ 12" c/c	M	12- AS529E (T) @ 12" c/c
	24- AS805E (B) @ 12" c/c		24- AS812E (B) @ 6" c/c
O	52- AS515E (T&B) @ 12" c/c	N	9- AS530E (T&B) @ 12" c/c
P	10- AS516E (T) @ 12" c/c	O	10- AS531E (T) @ 12" c/c
	20- AS806E (B) @ 6" c/c		20- AS813E (B) @ 6" c/c
		P	14- AS532E (T&B) @ 12" c/c



APPROACH SLAB - ABUTMENT #2
NOTE: SEE TABLE BELOW FOR BAR DESCRIPTIONS



APPROACH SLAB - ABUTMENT #1
NOTE: SEE TABLE BELOW FOR BAR DESCRIPTIONS

BAR SCHEDULE										
MARK	TYPE	NUMBER REQUIRED		LENGTH	DIMENSIONS					WEIGHT 602-02
		PHASE II	PHASE III		A	B	C	D	E	
APPROACH SLAB AT ABUTMENT #1										
AS501E	STR	-	20	9'-6"						198
AS502E	STR	-	10	19'-0" AVG.	14'-0" to	24'-0"				198
AS503E	STR	-	8	21'-6"						179
AS504E	STR	-	18	17'-3" AVG.	21'-0" to	13'-6"				324
AS505E	STR	-	12	20'-9" AVG.	14'-6" to	27'-0"				260
AS506E	STR	-	32	24'-9" AVG.	17'-6" to	32'-0"				826
AS507E	STR	-	19	31'-0" AVG.	25'-0" to	37'-0"				614
AS508E	STR	-	34	10'-6" AVG.	4'-0" to	17'-0"				372
AS509E	STR	34	-	17'-9"	-	-				629
AS510E	STR	15	-	29'-0" AVG.	20'-6" to	37'-6"				454
AS511E	STR	2	-	16'-0"	-	-				225
AS512E	STR	6	-	14'-6"	-	-				21
AS513E	STR	32	-	20'-3" AVG.	13'-6" to	27'-0"				676
AS514E	STR	12	-	23'-0"	16'-6" to	29'-6"				288
AS515E	STR	52	-	12'-0"	2'-0" to	22'-0"				651
AS516E	STR	10	-	20'-3" AVG.	15'-0" to	25'-6"				211
SUBTOTAL #5 BARS										6,196
AS801E	STR	-	20	9'-6"	-	-				507
AS802E	STR	-	24	20'-9" AVG.	14'-6" to	27'-0"				1,330
AS803E	STR	-	38	31'-0" AVG.	25'-0" to	37'-0"				3,145
AS804E	STR	30	-	29'-0" AVG.	20'-6" to	37'-6"				2,323
AS805E	STR	24	-	23'-0" AVG.	16'-6" to	29'-6"				1,474
AS806E	STR	20	-	20'-3" AVG.	15'-0" to	25'-6"				1,081
SUBTOTAL #8 BARS										9,860
TOTAL APPROACH SLAB 1 =										16,056

THE SUFFIX "E" IN THE MARK INDICATES EPOXY COATED REINFORCEMENT.

BAR SCHEDULE										
MARK	TYPE	NUMBER REQUIRED		LENGTH	DIMENSIONS					WEIGHT 602-02
		PHASE II	PHASE III		A	B	C	D	E	
APPROACH SLAB AT ABUTMENT #1										
AS517E	STR	-	48	11'-9" AVG.	2'-0"	21'-6"				588
AS518E	STR	-	10	24'-0" AVG.	15'-0" to	24'-0"				250
AS519E	STR	-	30	12'-6" AVG.	12'-6" to	24'-6"				391
AS520E	STR	-	12	29'-6" AVG.	15'-6" to	29'-6"				369
AS521E	STR	-	12	15'-0" AVG.	13'-0" to	17'-0"				188
AS522E	STR	-	28	18'-6"	-	-				540
AS523E	STR	-	19	24'-3" AVG.	14'-0" to	34'-6"				481
AS524E	STR	24	-	7'-6" AVG.	4'-0" to	11'-0"				188
AS525E	STR	2	-	19'-6"	-	-				41
AS526E	STR	20	-	21'-3" AVG.	13'-0" to	29'-6"				443
AS527E	STR	15	-	24'-0" AVG.	15'-6" to	32'-6"				375
AS528E	STR	20	-	16'-9" AVG.	13'-0" to	20'-6"				349
AS529E	STR	12	-	27'-9" AVG.	21'-6" to	34'-0"				347
AS530E	STR	9	-	21'-6"	-	-				202
AS531E	STR	10	-	21'-6" AVG.	15'-6" to	27'-6"				224
AS532E	STR	14	-	9'-6"	-	-				139
SUBTOTAL #5 BARS										5,115
AS807E	STR	-	20	19'-6" AVG.	15'-0" to	24'-0"				1,041
AS808E	STR	-	24	22'-6" AVG.	15'-6" to	29'-6"				1,442
AS809E	STR	-	38	24'-3" AVG.	14'-0" to	34'-6"				2,460
AS8010E	STR	4	-	19'-6"	-	-				208
AS8011E	STR	30	-	24'-0" AVG.	16'-6" to	32'-6"				1,922
AS8012E	STR	24	-	27'-9" AVG.	21'-6" to	34'-0"				1,778
AS8013E	STR	20	-	21'-6" AVG.	15'-6" to	27'-6"				1,148
SUBTOTAL #8 BARS										9,999
TOTAL APPROACH SLAB 2 =										15,114

ESTIMATED QUANTITIES *				
ITEM NO.	DESCRIPTION	UNIT	AS #1	AS #2
307-01	CLASS I AGGREGATE	CY	36	36
402-01 SG	HLBC SKID STONE & GRAVEL	TON	24	24
402-01 S	HLBC SKID SLAG ALT.	TON	22	22
409-02	BITUMINOUS MATERIAL	GAL	129	130
502-01 (12)	12" PCC APPROACH SLAB	SY	215	216
	EPOXY COATED REINFORCING STEEL	LB	16,056	15,114
	3/4" PREFORMED EXPANSION JOINT FILLER W/2" SILICONE JOINT SEALANT	LF	153	144

* NOTE: THESE QUANTITIES ARE INCLUDED IN ROADWAY ITEMS.

				WEST VIRGINIA DEPARTMENT OF TRANSPORTATION DIVISION OF HIGHWAYS KELLY CREEK BRIDGE #3886 REPLACEMENT			
				BAR SCHEDULE AND ESTIMATED QUANTITIES FOR APPROACH SLABS			
				Chapman Technical Group			
REVISION NUMBER	SHEET NUMBER	REVISIONS	DATE	BY			
SCALE: NONE				DATE: SEPTEMBER 1992			

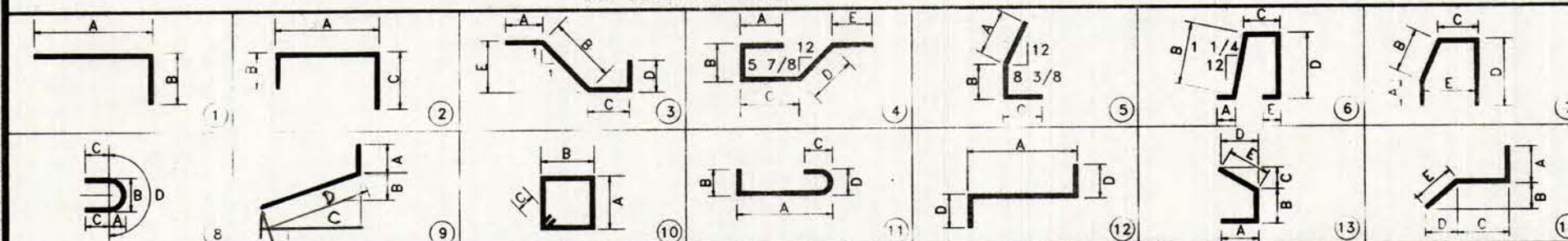
REINFORCING BAR SCHEDULE

MARK	TYPE	NUMBER REQUIRED			LENGTH	DIMENSIONS					WEIGHT	
		PHASE II	PHASE III	PHASE IV		A	B	C	D	E	602-01	602-02
S501E	STR	72	72	-	14'-1"						-	2,115
S502E	STR	12	6	-	14'-3"						-	268
S503E	5	210	209	-	3'-10 1/4"	2'-4"	8 1/4"	10"			-	1,684
S504E	6	210	209	-	9'-5 3/8"	1'-9"	3'-1 1/8"	8 1/4"	3'-1"	10"	-	4,129
S505E	STR	-	6	-	13'-1"						-	82
S506E	12	-	-	408	3'-2"	1'-2"	1'-0"				-	1,348
S507E	STR	-	-	204	3'-0"						-	638
S508E	4	106	132	-	8'-0"	1'-0"	11"	1'-6"	2'-1"	2'-6"	-	1,986
SUBTOTAL #5											0	12,250
S601E	STR	504	504	138	30'-0"						-	51,639
S602E	STR	84	84	23	35'-0"						-	10,041
S603E	STR	512	512	-	39'-2"						-	60,240
S604E	STR	2 SETS OF 63	2 SETS OF 63	-	19'-9" AVG.	3'-6" TO 36'-0"	STEP 6 5/16"				-	7,476
S605E	STR	-	2 SETS OF 67	-	20'-8 1/2" AVG.	2'-11" TO 38'-6"	STEP 6 1/2"				-	4,168
S606E	STR	2	2	-	2'-6"						-	15
S607E	STR	16	16	-	30'-0"						-	1,442
S608E	STR	-	16	-	33'-6"						-	805
S609E	STR	16	-	-	42'-3"						-	1,015
S610E	STR	2 SETS OF 69	-	-	20'-8" AVG.	2'-10" TO 38'-6"	STEP 6 3/16"				-	4,284
S611E	STR	-	-	622	7'-8"						-	7,163
S612E	STR	-	-	4 SETS OF 9	5'-1 1/2"	3'-0" TO 7'-3"	STEP 5 11/16"				-	277
S613E	STR	28	28	4	41'-0"						-	3,695
SUBTOTAL #6											0	152,260
TOTAL DECK											0	164,510

REINFORCING BAR SCHEDULE

MARK	TYPE	NUMBER REQUIRED				LENGTH	DIMENSIONS					WEIGHT	
		PIER 1N	PIER 1S	PIER 2N	PIER 2S		A	B	C	D	E	602-01	602-02
P501	STR	14	14	14	14	48'-1"						2,808	-
P502	STR	65	65	65	65	10'-1"						2,734	-
P503	STR	40	40	40	40	37'-6"						6,258	-
P504	8	40	40	40	40	8'-5 3/4"	1'-7"	3'-2"	1'-9"	4'-11 3/4"		1,415	-
P505	STR	2	2	2	2	41'-7"						347	-
P506	STR	2	2	2	2	48'-7"						405	-
P507	STR	10	10	10	10	54'-7"						2,277	-
P508	9	8	8	8	8	9'-10"	1'-0"	2'-5 1/8"	8'-6"	8'-10"		328	-
P509	10	2	2	2	2	17'-0 3/4"	4'-11 3/8"	3'-2"	5"			142	-
P510	10	2	2	2	2	18'-0 1/4"	5'-5 1/8"	3'-2"	5"			150	-
P511	10	2	2	2	2	18'-11 3/4"	5'-10 7/8"	3'-2"	5"			158	-
P512	10	2	2	2	2	19'-11 1/4"	6'-4 5/8"	3'-2"	5"			166	-
P513	11	247	247	247	247	4'-8"	3'-2"	10"	5"	5"		4,809	-
P514	2	8	8	-	-	7'-10"	2'-4"	3'-2"	2'-4"			131	-
P515	STR	4	4	-	-	15'-6"						130	-
P516	2	39	39	39	39	19'-2"	8'-0"	3'-2"	8'-0"			3,119	-
SUBTOTAL #5											25,377	-	-
P701	STR	88	88	88	88	26'-3"						19,246	-
P702	1	88	88	88	88	6'-11"	5'-9"	1'-2"				4,976	-
SUBTOTAL #7											24,222	-	-
P801	STR	14	14	14	14	48'-1"						7,189	-
P802	STR	59	59	59	59	10'-1"						6,354	-
SUBTOTAL #8											13,543	-	-
P1001	STR	8	8	8	8	10'-1"						1,388	-
P1002	1	14	14	14	14	31'-0"	13'-6"	4'-6"				7,470	-
SUBTOTAL #10											8,858	-	-
TOTAL PIERS											72,000	0	-

BAR BENDING DIAGRAM



ABUTMENT #1

MARK	TYPE	NUMBER REQUIRED			LENGTH	DIMENSIONS					WEIGHT	
		PHASE II	PHASE III			A	B	C	D	E	602-01	602-02
A501	2	114	124		11'-3"	6'-3"	2'-6"	2'-6"			2,793	-
A502	1	114	124		8'-0"	7'-0"	1'-0"				1,986	-
A503E	2	57			12'-11"	4'-5"	4'-3"	4'-3"			-	768
A504E	2	53	65		10'-1"	0'-6"	3'-4"	6'-3"			-	1,241
A506	STR	32	34		30'-0"						2,605	-
A506E	STR	10	10		30'-0"						-	626
A511	9	20	31		6'-9"	2'-6"	2'-4"	3'-0"	4'-3"		359	-
A511E	9	2	2		6'-9"	2'-6"	2'-4"	3'-0"	4'-3"		-	28
A512	1	4			7'-0"	3'-0"	4'-0"				29	-
A513	1	4			5'-0"	2'-6"	2'-6"				21	-
A513E	1	1			5'-0"	2'-6"	2'-6"				-	5
A514E	13	10			6'-4"	2'-0"	2'-0"	2'-0"	3'-0"	2'-4"	-	66
A515E	14		5		7'-10"	2'-10"	4'-8"	2'-0"	2'-0"	3'-0"	-	41
A517	STR	32			30'-5"						1,015	-
A517E	STR	10			30'-5"						-	317
A518	STR		18		35'-10"						673	-
A518E	STR		10		35'-10"						-	374
A519	STR		20		10'-3"						214	-
A520	1		7		6'-0"	3'-0"	3'-0"				44	-
A521E	2		62		16'-11"	4'-5"	6'-3"	6'-3"			-	1,094
A522	STR		2		9'-3"						19	-
A523	STR		14		8'-3"						120	-
A524E	STR		18		5'-9"						-	108
A525	STR		1		11'-4"						12	-
A526	STR		8		15'-6"						129	-
A527E	7		4		9'-8"	2'-6"	2'-6"	0'-8"	5'-0"	1'-5"	-	40
A528E	5		4		4'-10"	2'-4"	2'-6"				-	20
A529	STR		18		38'-6"						722	-
TOTAL #5											10,741	4,728
A601E	2	53	65		12'-3"	1'-3"	6'-0"	5'-0"			-	1,196
TOTAL #6											-	1,196
TOTAL ABUT. #1											10,741	5,924

ABUTMENT #2

MARK	TYPE	NUMBER REQUIRED			LENGTH	DIMENSIONS					WEIGHT	
		PHASE II	PHASE III			A	B	C	D	E	602-01	602-02
A501	2	112	130		11'-3"	6'-3"	2'-6"	2'-6"			2,840	-
A504E	2	56	61		10'-1"	0'-6"	3'-4"	6'-3"			-	1,230
A506	STR	28	26		30'-0"						1,690	-
A506E	STR	10	10		30'-0"						-	626
A507	STR		26		35'-9"						969	-
A507E	STR		10		35'-9"						-	373
A508	STR	5			29'-9"						155	-
A509	STR	10			32'-3"						336	-
A510	STR	15			34'-9"						544	-
A510E	STR	10			34'-9"						-	362
A511	9	21	20		6'-9"	2'-6"	2'-4"	3'-0"	4'-3"		289	-
A511E	9	6			6'-9"	2'-6"	2'-4"	3'-0"	4'-3"		-	-
A512	1		4		7'-0"	3'-0"	4'-0"				29	-
A513E	1		4		5'-0"	2'-6"	2'-6"				21	-
A514E	13		10		6'-3"	2'-0"	2'-0"	2'-0"	3'-0"	2'-4"	-	65
A515E	14	5			7'-10"	2'-10"	4'-8"	2'-0"	2'-0"	3'-0"	-	41
A516	1	112	130		7'-0"	6'-0"	1'-0"				1,767	-
A529E	2	65	65		10'-5"	4'-5"	3'-9"	3'-9"			-	1,412
A530E	2	56			11'-11"	4'-5"	3'-9"	3'-9"			-	696
TOTAL #5											8,640	4,805
A601E	2	56	61		12'-3"	1'-3"	6'-0"	5'-0"			-	1,196
TOTAL #6											0	2,153
TOTAL ABUT. #2											8,640	6,958

NOTES: FIGURES IN CIRCLE SHOW BAR TYPE.
ALL DIMENSIONS ARE FROM OUT TO OUT OF BARS.
NO CORRECTION IN LENGTH FOR BEND CURVATURE IS TO BE MADE.
FOR STANDARD HOOK DIMENSIONS SEE A.C.I. MANUAL OF CONCRETE PRACTICE
FOR DETAILING CONCRETE STRUCTURES (A.C.I. 315).
FIRST DIGIT FOLLOWING PREFIX INDICATES BAR SIZE. EXAMPLE: S501 = #5, A801 = #8.
THE SUFFIX "E" IN THE MARK INDICATES EPOXY COATED REINFORCEMENT.

WEST VIRGINIA DEPARTMENT OF TRANSPORTATION
DIVISION OF HIGHWAYS
KELLY CREEK BRIDGE #3886 REPLACEMENT

REINFORCING BAR SCHEDULE

Chapman Technical Group

REVISIONS
DATE: SEPTEMBER 1992
SCALE: AS SHOWN

WINGWALL #1 (PHASE III CONSTRUCTION)										
MARK	TYPE	NUMBER REQUIRED	LENGTH	DIMENSIONS					WEIGHT	
				A	B	C	D	E	602-01	602-02
W501	STR	106	19'-6"						2,156	-
W501E	STR	24	19'-6"						-	488
W502	STR	44	5'-0"						229	-
W503	2	40	14'-0"	8'-0"	3'-0"	3'-0"			584	-
W504	STR	20	14'-2"						296	-
W505E	7	60	9'-8"	2'-0"	2'-6"	0'-8"	4'-6"	1'-5"	-	605
W506E	5	60	4'-10"	2'-4"	2'-6"	-			-	302
W507	2	40	12'-0"	7'-0"	2'-6"	2'-6"			501	-
W508	2	40	8'-0"	3'-0"	2'-6"	2'-6"			334	-
W509	STR	20	10'-6"						219	-
W510	STR	20	4'-10"						101	-
TOTAL #5									4,420	1,395
W601	STR	20	14'-8"						441	-
W602	STR	20	10'-6"						315	-
W603	STR	20	4'-10"						145	-
TOTAL #6									901	-
TOTAL WINGWALL #1									5,321	1,395

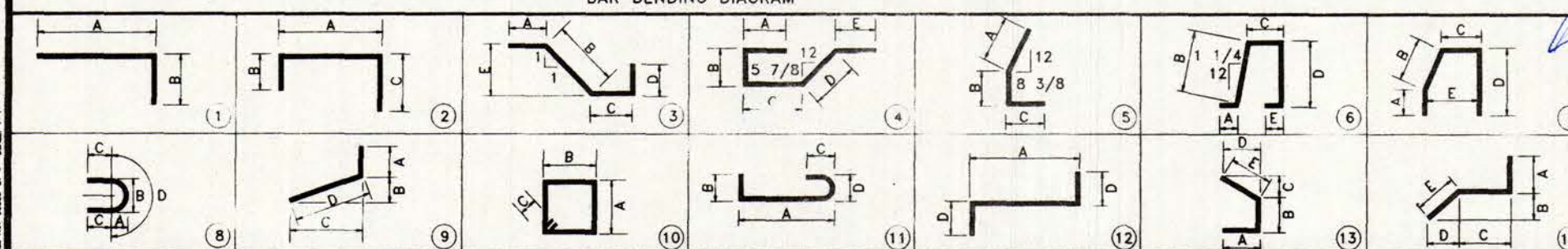
WINGWALL #3 (PHASE III CONSTRUCTION)										
MARK	TYPE	NUMBER REQUIRED	LENGTH	DIMENSIONS					WEIGHT	
				A	B	C	D	E	602-01	602-02
W505E	7	11	10'-8"	2'-6"	2'-6"	0'-8"	5'-0"	1'-5"	-	122
W506E	5	11	4'-10"	2'-4"	2'-6"				-	55
W508	2	22	8'-0"	3'-0"	2'-6"	2'-6"			184	-
W512	STR	22	5'-0"						115	-
W514	STR	11	7'-8"						88	-
W517	STR	20	12'-0"						250	-
W521	STR	8	10'-2"						85	-
W521E	STR	8	10'-2"						-	85
TOTAL #5									722	262
W606	STR	11	7'-8"						127	-
TOTAL #6									127	-
TOTAL WINGWALL #3									849	262

WINGWALL #2 (PHASE II CONSTRUCTION)										
MARK	TYPE	NUMBER REQUIRED	LENGTH	DIMENSIONS					WEIGHT	
				A	B	C	D	E	602-01	602-02
W505E	7	24	10'-8"	2'-6"	2'-6"	0'-8"	5'-0"	1'-5"	-	267
W506E	5	24	4'-10"	2'-4"	2'-6"				-	121
W508	2	48	8'-0"	3'-0"	2'-6"	2'-6"			400	-
W512	STR	48	5'-0"						250	-
W513	STR	24	10'-0"						250	-
W516	STR	36	18'-10"						707	-
W516E	STR	8	18'-10"						-	157
W519	STR	10	6'-6"						-	-
W519E	STR	8	6'-6"						-	54
W522	STR	26	8'-1"						219	-
TOTAL #5									1,826	599
W605	STR	24	10'-0"						360	-
TOTAL #6									360	-
TOTAL WINGWALL #2									2,186	599

WINGWALL #4 (PHASE II CONSTRUCTION)										
MARK	TYPE	NUMBER REQUIRED	LENGTH	DIMENSIONS					WEIGHT	
				A	B	C	D	E	602-01	602-02
W505E	7	17	10'-8"	2'-6"	2'-6"	0'-8"	5'-0"	1'-5"	-	189
W506E	5	17	4'-10"	2'-4"	2'-6"				-	86
W508	2	42	8'-0"	3'-0"	2'-6"	2'-6"			350	-
W512	STR	34	5'-0"						177	-
W515	STR	17	9'-1"						161	-
W518	STR	22	20'-6"						470	-
W520	STR	10	15'-6"						162	-
W520E	STR	8	15'-6"						-	129
TOTAL #5									1,320	404
W607	STR	17	9'-1"						232	-
TOTAL #6									232	-
TOTAL WINGWALL #4									1,552	404

NOTES:
 FIGURES IN CIRCLE SHOW BAR TYPE.
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 FIRST DIGIT FOLLOWING PREFIX INDICATES BAR SIZE. EXAMPLE: S501 = #5, A801 = #8.
 THE SUFFIX "E" IN THE MARK INDICATES EPOXY COATED REINFORCEMENT.

BAR BENDING DIAGRAM



SCALE: AS SHOWN

DATE: SEPTEMBER 1992

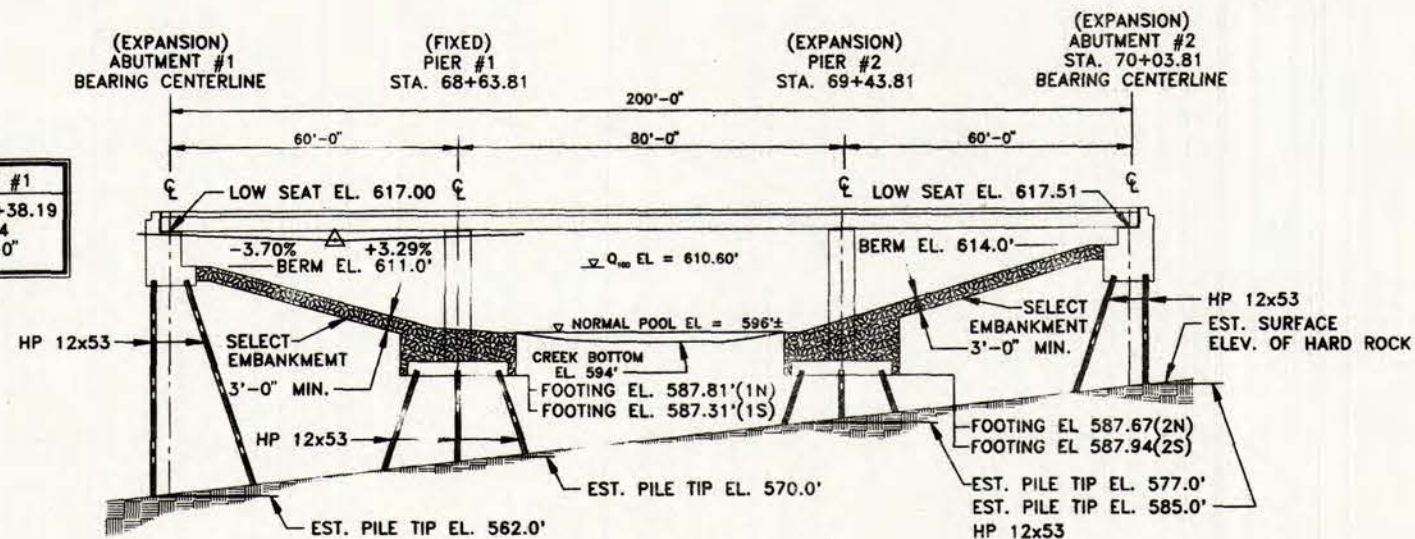
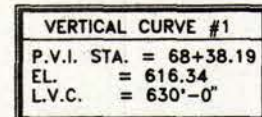
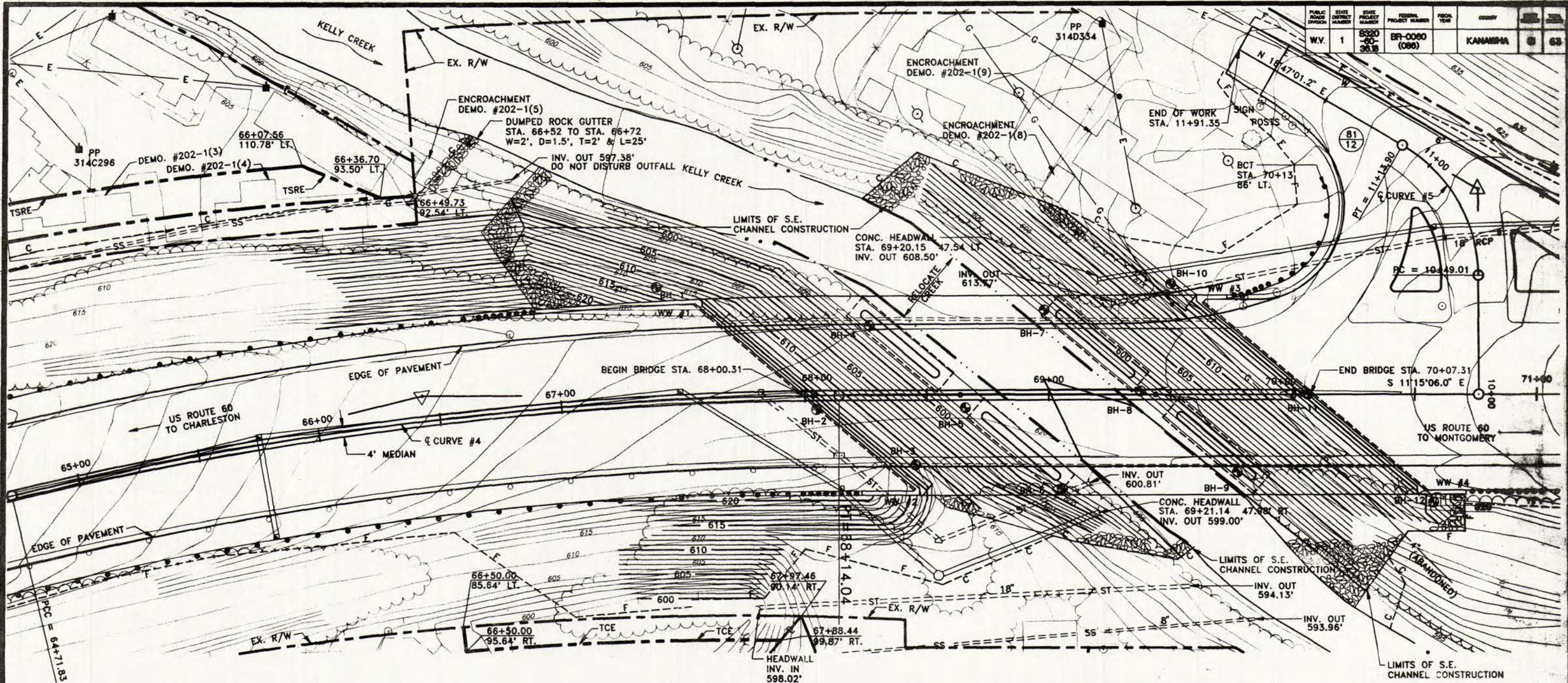
WEST VIRGINIA DEPARTMENT OF TRANSPORTATION
 DIVISION OF HIGHWAYS
 KELLY CREEK BRIDGE #3886 REPLACEMENT

REINFORCING BAR SCHEDULE

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PUBLIC ROADS DIVISION	STATE DISTRICT NUMBER	STATE PROJECT NUMBER	FEDERAL PROJECT NUMBER	FISCAL YEAR	COUNTY	1962	1963
W.V.	1	8320 -60 36.18	BF-0060 (066)		KANAWHA	6	63



CENTERLINE ELEVATION

NOTE:
BASED ON BOULDERS ENCOUNTERED DURING
SUBSURFACE INVESTIGATION, PILE LOCATIONS
AT PIER #2 SHALL BE PREDRILLED AND DRIVEN.
MINIMUM PILE LENGTH IS 10 FEET.

Q_{10} EL = 602.70'
 Q_{20} EL = 607.30'
 Q_{30} EL = 610.60'
 Q_{40} EL = 625.90'

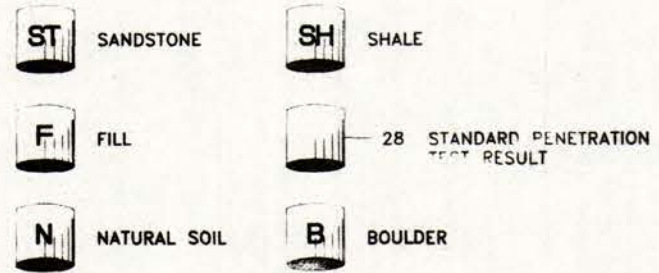
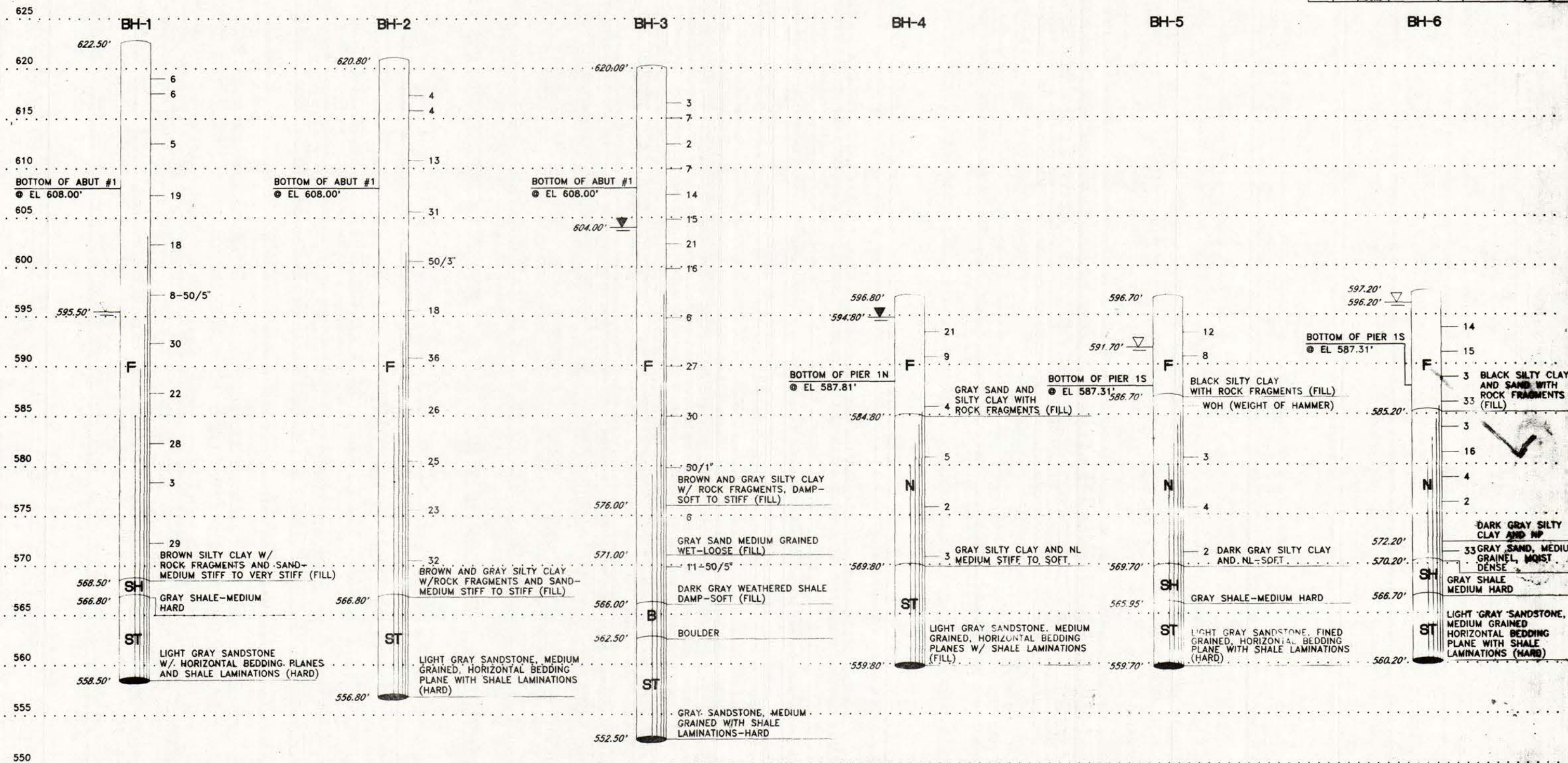
NOTE:
BACKWATER FROM THE
KANAWHA RIVER GOVERNS
ALL FLOOD ELEVATIONS

WEST VIRGINIA DEPARTMENT OF TRANSPORTATION
DIVISION OF HIGHWAYS
KELLY CREEK BRIDGE #3886 REPLACEMENT

SITUATION PLAN

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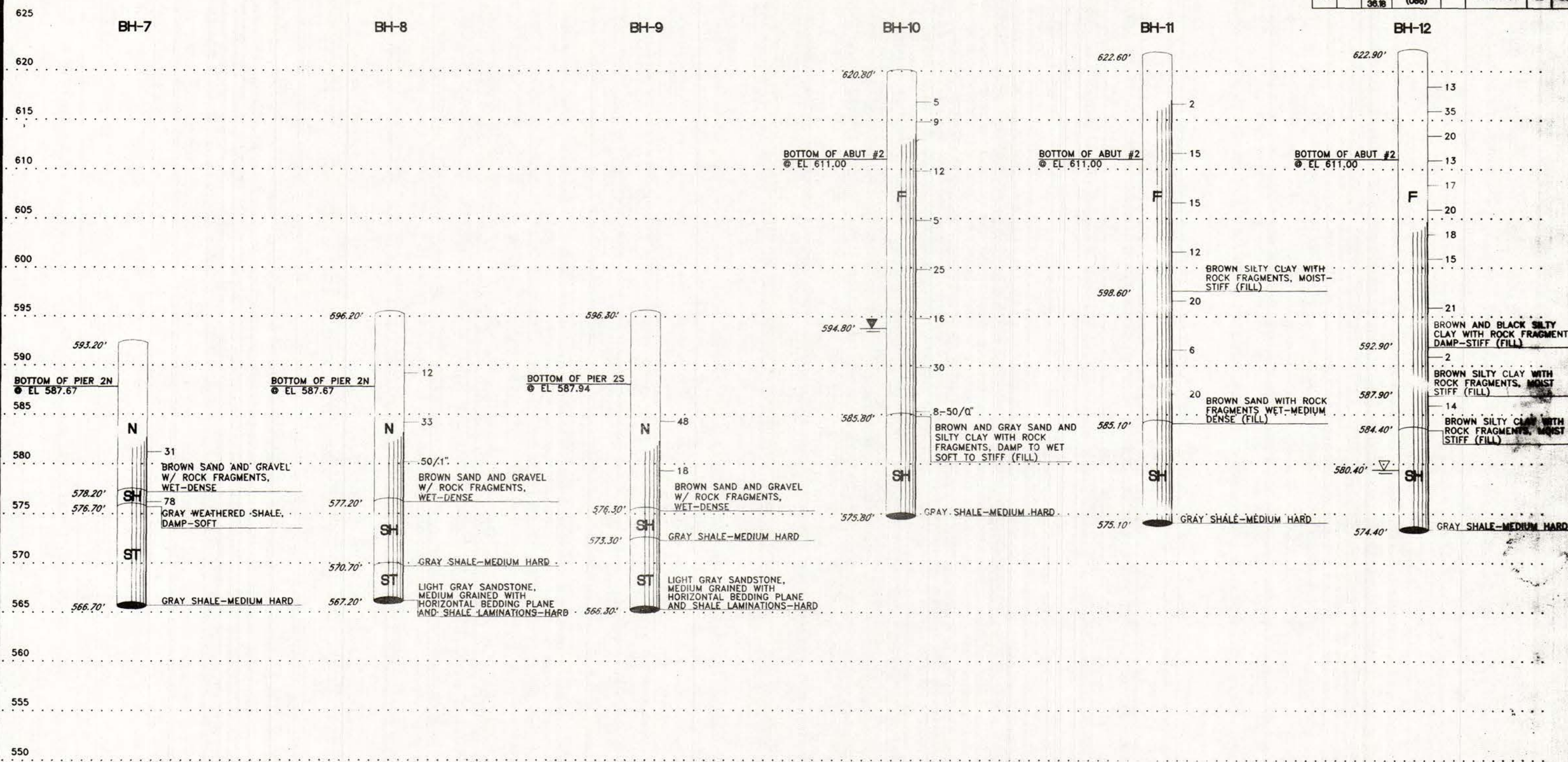
WATER TABLE 24 HOURS
 WATER TABLE IMMEDIATE



REVISION NUMBER	SHEET NUMBER	REVISIONS	DATE	BY

SCALE: AS SHOWN DATE: SEPTEMBER 1992

PUBLIC ROADS DIVISION	STATE DISTRICT NUMBER	STATE PROJECT NUMBER	FEDERAL PROJECT NUMBER	FISCAL YEAR	COUNTY	SHEET NUMBER	TOTAL SHEETS
W.V.	1	5320-80-38.18	BF-0000 (086)		KANAWHA	88	89



ST SANDSTONE

SH SHALE

F FILL

B BOULDER

N NATURAL SOIL

28 STANDARD PENETRATION TEST RESULT

WATER TABLE

24 HOURS

WATER TABLE

IMMEDIATE

GRAPHIC SCALE

0

1

2

3

4

5

10

1"=5' (IN FEET)

REVISION NUMBER

SHEET NUMBER

REVISIONS

DATE

BY

SCALE: AS SHOWN

DATE: SEPTEMBER 1992

WEST VIRGINIA DEPARTMENT OF TRANSPORTATION

DIVISION OF HIGHWAYS

KELLY CREEK BRIDGE #3886 REPLACEMENT

BORE HOLE SECTIONS

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